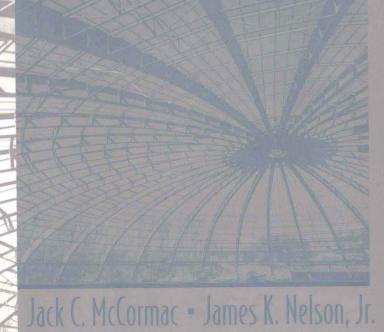
International Edition

# Structural Steel Design

LRFD Method

Third Edition





# Structural Steel Design: LRFD Method

# THIRD EDITION

Jack C. McCormac
Clemson University, Clemson, South Carolina
and

James K. Nelson, Jr. Western Michigan University, Kalamazoo, Michigan



Pearson Education International

If you purchased this book within the United States or Canada you should be aware that it has been wrongfully imported without the approval of the Publisher or the Author.



© 2003 by Pearson Education, Inc. Pearson Education, Inc. Upper Saddle River, NJ 07458

All rights reserved. No part of this book may be reproduced in any form or by any means, without permission in writing from the publisher.

The author and publisher of this book have used their best efforts in preparing this book. These efforts include the development, research, and testing of the theories and programs to determine their effectiveness. The author and publisher make no warranty of any kind, expressed or implied, with regard to these programs or the documentation contained in this book. The author and publisher shall not be liable in any event for incidental or consequential damages in connection with, or arising out of, the furnishing, performance, or use of these programs.

Printed in the United States of America 10987654321

ISBN 0-13-128608-0

Pearson Education Ltd., London
Pearson Education Australia Pty. Ltd., Sydney
Pearson Education Singapore, Pte. Ltd.
Pearson Education North Asia Ltd., Hong Kong
Pearson Education Canada, Inc., Toronto
Pearson Educación de Mexico, S.A. de C.V.
Pearson Education—Japan, Tokyo
Pearson Education Malaysia, Pte. Ltd.
Pearson Education, Inc., Upper Saddle River, New Jersey

# **Preface**

The authors' major objective in preparing this new edition was to update the text to conform to both the 1999 Load and Resistance Factor (LRFD) Design Specification and the 2002 edition of the LRFD Manual of Steel Construction.

Among the several changes made in the new specification and included in the steel manual are the following:

- 1. The inclusion of data and equations in both U.S. customary units and metric units
- 2. The introduction of two new important ASTM steels, A913 and A992.
- 3. A few revisions in bolt criteria.
- 4. Revised design procedure for fatigue loadings.
- 5. A new section concerning the evaluation of existing structures.

In addition to the revisions in the Specification, several changes and additions have been made in the text concerning the enclosed computer programs. First, the program previously named INSTEP has been updated to the new specifications and has been changed from a DOS format to a Windows format. It is now named INSTEP32. Though the program was written specifically to solve many of the textbook-type problems presented in this book, it has often been used by professional engineers in their design practices.

Despite the practical applications of INSTEP32 and its value in teaching steel design, the authors feel that many professors would like their students to have some experience with at least one of the major commercial steel-design programs on the market today, in hopes that such experience would enable students to cross the bridge more quickly between the classroom and actual design practice. As a result, a student version of SAP 2000 has been included with INSTEP32 on the enclosed disk and presented herein. Use of this software will also enable the student to better understand the relationship between analysis and design.

Chapter 20, a new chapter in the text, presents an introduction to the subject of systems design. In this regard, recent requirements for "capstone" courses in our engineering schools have made the subject of "open-ended" problems a serious component of design studies. Therefore, the topic of systems design along with "open-ended" problems and an introduction to SAP 2000 are included in Chapter 20.

The authors wish to thank the following persons who reviewed this edition: Robert Abendroth, Daniel G. Linzell, Rolla Idriss, W. H. Walker, and Ahmad M. Itani.

They also thank the reviewers and users of the previous editions of this book for their suggestions, corrections, and criticisms. They are always grateful to anyone who takes the time to contact them concerning any part of their book.

Jack C. McCormac James K. Nelson

# **Contents**

Preface		iii
CHAPTER 1 Introdu	1	
1.1	Advantages of Steel as a Structural Material	1
1.2	Disadvantages of Steel as a Structural Material	3
1.3	Early Uses of Iron and Steel	4
1.4	Steel Sections	6
1.5	Metric Units	10
1.6	Cold-Formed Light-Gage Steel Shapes	11
1.7	Stress-Strain Relationships in Structural Steel	12
1.8	Modern Structural Steels	16
1.9	Uses of High-Strength Steels	22
1.10	Measurement of Toughness	23
1.11	Jumbo Sections	24
1.12	Lamellar Tearing	25
1.13	Furnishing of Structural Steel	25
1.14	The Work of the Structural Designer	28
1.15	Responsibilities of the Structural Designer	28
1.16	Economical Design of Steel Members	29
1.17	Failure of Structures	33
1.18	Handling and Shipping Structural Steel	34
1.19	Calculation Accuracy	34
1.20	Computers and Structural Steel Design	34
1.21	Computer-Aided Design in this Text	35
CHAPTER 2 Specific	36	
2.1	Specifications and Building Codes	36
2.2	Loads	38
2.3	Dead Loads	38
2.4	Live Loads	39
2.5	Environmental Loads	42

# vi Contents

	2.6	Load and Resistance-Factor Design	40
	2.7	Load Factors	49
	2.8	Resistance Factors	53
	2.9	Discussion of Sizes of Load and Resistance Factors	54
	2.10	Reliability and the LRFD Specification	54
	2.11	Advantages of LRFD	57
	2.12	Computer Example	58
		Problems	60
CHAPTER	3 Analy	sis of Tension Members	61
	3.1	Introduction	61
	3.2	Design Strength of Tension Members	64
	3.3	Net Areas	65
	3.4	Effect of Staggered Holes	68
	3.5	Effective Net Areas	73
	3.6	Connecting Elements for Tension Members	80
	3.7	Block Shear	81
	3.8	Computer Example	87
		Problems	88
CHAPTER	4 Design	n of Tension Members	98
	4.1	Selection of Sections	98
	4.2	Built-Up Tension Members	104
	4.3	Rods and Bars	108
	4.4	Pin-Connected Members	112
	4.5	Design for Fatigue Loads	115
	4.6	Computer Example	117
		Problems	118
CHAPTER	5 Introd	luction to Axially Loaded Compression Members	122
	5.1	General	122
	5.2	Residual Stresses	125
	5.3	Sections Used for Columns	126
	5.4	Development of Column Formulas	130
	5.5	The Euler Formula	131
	5.6	End Restraint and Effective Lengths of Columns	132
	5.7	Stiffened and Unstiffened Elements	136
	5.8	Long, Short, and Intermediate Columns	140
	5.9	Column Formulas	140
	5.10	Maximum Slenderness Ratios	142
	5.11	Example Problems	142
	5.12	Computer Example	147
		Problems	148

		Contents	VII
CHAPTER 6	Design	of Axially Loaded Columns	154
	6.1	Introduction	154
	6.2	LRFD Design Tables	157
	6.3	Column Splices	161
	6.4	Built-Up Columns	164
	6.5	Built-Up Columns with Components in Contact with Each Other	164
	6.6	Connection Requirements for Built-Up Columns whose Components are in Contact with Each Other	166
	6.7	Built-Up Columns with Components not in Contact with Each Other	171
	6.8	Introductory Remarks Concerning Flexural-Torsional Buckling of	
		Compression Members	176
	6.9	Single-Angle Compression Members	177
	6.10	Computer Example	177
		Problems	178
CHAPTER 7	Design	of Axially Loaded Compression Members, Continued	182
	7.1	Further Discussion of Effective Lengths	182
	7.2	Frames Meeting Alignment Chart Assumptions	187
	7.3	Frames not Meeting Alignment Chart Assumptions as to Joint	
		Rotations	189
	7.4	Stiffness-Reduction Factors	192
	7.5	Columns Leaning on Each Other for In-Plane Design	195
	7.6	Base Plates for Concentrically Loaded Columns	199
	7.7	Computer Example	209
		Problems	211
CHAPTER 8	Introdu	action to Beams	215
	8.1	Types of Beams	215
	8.2	Sections Used as Beams	215
	8.3	Bending Stresses	216
	8.4	Plastic Hinges	217
	8.5	Elastic Design	218
	8.6	The Plastic Modulus	218
	8.7	Theory of Plastic Analysis	221
	8.8	The Collapse Mechanism	222
	8.9	The Virtual-Work Method	223
	8.10	Location of Plastic Hinge for Uniform Loadings	227
	8.11	Continuous Beams	228
	8.12	Building Frames	230
		Problems	232

CHAPTER 9	Design o	of Beams for Moments	241
	9.1	Introduction	241
	9.2	Yielding Behavior—Full Plastic Moment, Zone 1	244
	9.3	Design of Beams, Zone 1	246
	9.4	Lateral Support of Beams	253
	9.5	Introduction to Inelastic Buckling, Zone 2	255
	9.6	Moment Capacities, Zone 2	258
	9.7	Elastic Buckling, Zone 3	259
	9.8	Design Charts	262
	9.9	Noncompact Sections	265
	9.10	Computer Example	267
		Problems	268
CHAPTER 10	Design	of Beams—Miscellaneous Topics	274
	10.1	Design of Continuous Beams	274
	10.2	Shear	276
	10.3	Deflections	282
	10.4	Webs and Flanges with Concentrated Loads	287
	10.5	Unsymmetrical Bending	294
	10.6	Design of Purlins	296
	10.7	The Shear Center	300
	10.8	Beam Bearing Plates	305
	10.9	Computer Example	308
		Problems	309
CHAPTER 11	Bendin	ng and Axial Force	316
	11.1	Occurence	316
	11.2	Members Subject to Bending and Axial Tension	317
	11.3	Computer Examples for Members Subject to Bending and Axial	
	33.4	Tension	320
	11.4	First-Order and Second-Order Moments for Members Subject to Axial Compression and Bending	322
	11.5	Magnification Factors	323
	11.6	Moment Magnification or $C_m$ Factors	325
	11.7	Review of Beam–Columns in Braced Frames	329
	11.8	Review of Beam–Columns in Unbraced Frames	334
	11.9	Design of Beam–Columns—Braced or Unbraced	336
	11.10	Computer Examples for Members Subject to Bending and Axial	550
		Compression	343
		Problems	345

		Contents	ix
CHAPTER 12	Bolted	Connections	349
	12.1	Introduction	349
	12.2	Types of Bolts	349
	12.3	History of High-Strength Bolts	350
	12.4	Advantages of High-Strength Bolts	351
	12.5	Snug-Tight, Pretensioned, and Slip-Critical Bolts	351
	12.6	Methods for Fully Tensioning High-Strength Bolts	354
	12.7	Slip-Resistant Connections and Bearing-Type Connections	357
	12.8	Mixed Joints	358
	12.9	Sizes of Bolt Holes	359
	12.10	Load Transfer and Types of Joints	360
	12.11	Failure of Bolted Joints	363
	12.12	Spacing and Edge Distances of Bolts	363
	12.13	Bearing-Type Connections—Loads Passing Through	
		Center of Gravity of Connections	367
	12.14	Slip-Critical Connections—Loads Passing Through	
		Center of Gravity of Connections	376
	12.15	Computer Example	382
		Problems	383
CHAPTER 13	Eccent	trically Loaded Bolted Connections and Historical Notes on Rivets	390
	13.1	Bolts Subjected to Eccentric Shear	390
	13.2	Bolts Subjected to Shear and Tension	403
	13.3	Tension Loads on Bolted Joints	406
	13.4	Prying Action	408
	13.5	Historical Notes on Rivets	413
	13.6	Types of Rivets	414
	13.7	Strength of Riveted Connections—Rivets in Shear	415
	13.8	Computer Example	418
		Problems	419
CHAPTER 14	Welde	d Connections	429
	14.1	General	429
	14.2	Advantages of Welding	430
	14.3	American Welding Society	431
	14.4	Types of Welding	431
	14.5	Prequalified Welding	435
	14.6	Welding Inspection	435
	14.7	Classification of Welds	437
	14.8	Welding Symbols	440
	140	Granya Walds	440

443

14.10

Fillet Welds

# x Contents

	14.11	Strength of Welds	445
	14.12	LRFD Requirements	445
		Design of Simple Fillet Welds	450
		Design of Fillet Welds for Truss Members	455
		Shear and Torsion	460
	14.16	Shear and Bending	467
	14.17	Design of Moment-Resisting Connections	469
	14.18	Full-Penetration and Partial-Penetration Groove Welds	471
	14.19	Computer Examples	474
		Problems	475
CHAPTER 15	Buildin	g Connections	486
	15.1	Selection of Type of Fastener	486
	15.2	Type of Beam Connections	487
	15.3	Standard Bolted Beam Connections	493
	15.4	LRFD Manual Standard Connection Tables	497
	15.5	Designs of Standard Bolted Framed Connections	498
	15.6	Designs of Standard Welded Framed Connections	500
	15.7	Single-Plate or Shear Tab Framing Connections	502
	15.8	End-Plate Shear Connections	505
	15.9	Designs of Welded Seated Beam Connections	505
	15.10	Stiffened Seated Beam Connections	508
	15.11	Design of Moment-Resisting Connections	509
	15.12	Column Web Stiffeners	510
	15.13	Connection Design Aids—Handbooks and Computer Programs	513
		Problems	514
CHAPTER 16	Compo	site Beams	516
	16.1	Composite Construction	516
	16.2	Advantages of Composite Construction	518
	16.3	Discussion of Shoring	518
	16.4	Effective Flange Widths	520
	16.5	Shear Transfer	521
	16.6	Partially Composite Beams	523
	16.7	Strength of Shear Connections	524
	16.8	Number, Spacing, and Cover Requirements for Shear Connections	525
	16.9	Moment Capacity of Composite Sections	527
	16.10	Deflections	532
	16.11	Design of Composite Sections	533
	16.12	Continuous Composite Sections	541
	16.13	Design of Concrete-Encased Sections	542
		Problems	545

			==0
CHAPTER 17	Compo	site Columns	550
	17.1	Introduction	550
	17.2	Advantages of Composite Columns	551
	17.3	Disadvantages of Composite Columns	553
	17.4	Lateral Bracing	553
	17.5	Specifications for Composite Columns	554
	17.6	Axial Design Strengths of Composite Columns	555
	17.7	LRFD Tables	558
	17.8	Flexural Design Strengths of Composite Columns	562
	17.9	Axial Load and Bending Equation	562
	17.10	Design of Composite Columns Subject to Axial Load and Bending	563
	17.11	Load Transfer at Footings and Other Connections	565
		Problems	565
CHAPTER 18	Built-U	Jp Beams, Built-Up Wide-Flange Sections, and Plate Girders	568
	18.1	Cover-Plated Beams	568
	18.2	Built-Up Wide Flange Sections	570
	18.3	Introduction to Plate Girders	575
	18.4	Plate Girder Proportions	577
	18.5	Detailed Proportions of Webs	580
	18.6	Design of Plate Girders with Slender Webs, but with Full Lateral	
		Bracing for their Compact Compression Flanges	582
	18.7	Design of Plate Girders with Noncompact Flanges and without Full Lateral Bracing for Compression Flanges	585
	18.8	Design of Stiffeners	590
	18.9	Flexure–Shear Interaction	594
	10.5	Problems	600
CHAPTER 19	Design	of Steel Buildings	601
CHAITER 13	-		
	19.1	Introduction to Low-Rise Buildings	601
	19.2	Types of Steel Frames Used for Buildings	601
	19.3	Common Types of Floor Construction	605
	19.4	Concrete Slabs on Open-Web Steel Joists	606
	19.5	One-Way and Two-Way Reinforced Concrete Slabs	609
	19.6	Composite Floors	611
	19.7	Concrete-Pan Floors	612
	19.8	Steel-Decking Floors	614
	19.9	Flat Slabs	614
	19.10	Precast Concrete Floors	615
	19.11	Types of Roof Construction	617
	19.12	Exterior Walls and Interior Partitions	618
	19.13	Fireproofing of Structural Steel	619

Contents

# xii Contents

	19.14	Introduction to High-Rise Buildings	620
	19.15	Discussion of Lateral Forces	621
	19.16	Types of Lateral Bracing	623
	19.17	Analysis of Buildings with Diagonal Wind Bracing for	
		Lateral Forces	628
	19.18	Moment-Resisting Joints	630
	19.19	Analysis of Buildings with Moment-Resisting Joints for	
		Lateral Loads	632
	19.20	Analysis of Buildings for Gravity Loads	636
	19.21	Design of Members	639
CHAPTER 20	Design	of Steel Building Systems	641
	20.1	Introduction	641
	20.2	Design of Structural Steel Building	646
	20.3	Loads Acting on the Structural Frame	648
	20.4	Preliminary Design and Analysis	653
	20.5	Review of the Results and Design Changes	655
	20.6	The Design Sketches	656
	20.7	Concluding Comments	657
		Problems	658
APPENDIX A	Allowa	able Stress Design	661
APPENDIX B	Deriva	tion of the Euler Formula	678
APPENDIX C	Slende	r Compression Elements	680
APPENDIX D	Flexur	al-Torsional Buckling of Compression Members	683
APPENDIX E	Momen	nt-Resisting Column Base Plates	689
APPENDIX F	Pondin	$\mathbf{g}$	697
GLOSSARY			701
INDEX			707

# CHAPTER 1

# Introduction to Structural Steel Design

## 1.1 ADVANTAGES OF STEEL AS A STRUCTURAL MATERIAL

A person traveling in the United States might quite understandably decide that steel was the perfect structural material. He or she would see an endless number of steel bridges, buildings, towers, and other structures. After seeing these numerous steel structures, this traveler might be surprised to learn that steel was not economically made in the United States until late in the nineteenth century, and the first wide-flange beams were not rolled until 1908.

The assumption of the perfection of this metal, perhaps the most versatile of structural materials, would appear to be even more reasonable when its great strength, light weight, ease of fabrication, and many other desirable properties are considered. These and other advantages of structural steel are discussed in detail in the paragraphs that follow.

# 1.1.1 High Strength

The high strength of steel per unit of weight means that the weight of structures will be small. This fact is of great importance for long-span bridges, tall buildings, and structures situated on poor foundations.

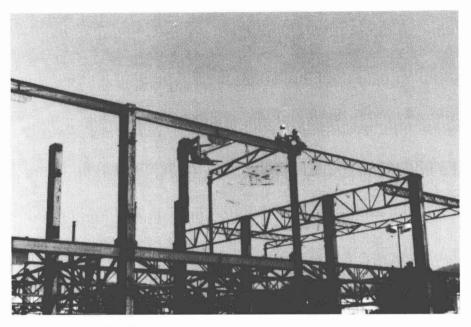
# 1.1.2 Uniformity

The properties of steel do not change appreciably with time, as do those of a reinforced-concrete structure.

# 1.1.3 Elasticity

Steel behaves closer to design assumptions than most materials because it follows Hooke's law up to fairly high stresses. The moments of inertia of a steel structure can be accurately calculated, while the values obtained for a reinforced-concrete structure are rather indefinite.





Erection of steel joists. (Courtesy of Vulcraft.)

### 1.1.4 **Permanence**

Steel frames that are properly maintained will last indefinitely. Research on some of the newer steels indicates that under certain conditions no painting maintenance whatsoever will be required.

### 1.1.5 Ductility

The property of a material by which it can withstand extensive deformation without failure under high tensile stresses is said to be its ductility. When a mild or low-carbon structural steel member is being tested in tension, a considerable reduction in cross section and a large amount of elongation will occur at the point of failure before the actual fracture occurs. A material that does not have this property is generally unacceptable and is probably hard and brittle, and it might break if subjected to a sudden shock.

In structural members under normal loads, high stress concentrations develop at various points. The ductile nature of the usual structural steels enables them to yield locally at those points, thus preventing premature failures. A further advantage of ductile structures is that when overloaded their large deflections give visible evidence of impending failure (sometimes jokingly referred to as "running time").

### 1.1.6 **Toughness**

Structural steels are tough—that is, they have both strength and ductility. A steel member loaded until it has large deformations will still be able to withstand large forces. This is a very important characteristic because it means that steel members can be subjected to large deformations during fabrication and erection without fracture—thus allowing them to be bent, hammered, sheared, and have holes punched in them without visible damage. The ability of a material to absorb energy in large amounts is called *toughness*.

# 1.1.7 Additions to Existing Structures

Steel structures are quite well suited to having additions made to them. New bays or even entire new wings can be added to existing steel frame buildings, and steel bridges may often be widened.

## 1.1.8 Miscellaneous

Several other important advantages of structural steel are as follows: (a) ability to be fastened together by several simple connection devices including welds and bolts, (b) adaptation to prefabrication, (c) speed of erection, (d) ability to be rolled into a wide variety of sizes and shapes as described in Section 1–4, (e) fatigue strength, (f) possible reuse after a structure is disassembled, and (g) scrap value, even though not reusable in its existing form. Steel is the ultimate recyclable material.

# 1.2 DISADVANTAGES OF STEEL AS A STRUCTURAL MATERIAL

In general, steel has the following disadvantages:

# 1.2.1 Maintenance Costs

Most steels are susceptible to corrosion when freely exposed to air and water and therefore must be painted periodically. The use of weathering steels, however, in suitable applications tends to eliminate this cost.

# 1.2.2 Fireproofing Costs

Although structural members are incombustible, their strength is tremendously reduced at temperatures commonly reached in fires when the other materials in a building burn. Many disastrous fires have occurred in empty buildings where the only fuel for the fires was the buildings themselves. Furthermore, steel is an excellent heat conductor—nonfireproofed steel members may transmit enough heat from a burning section or compartment of a building to ignite materials with which they are in contact in adjoining sections of the building. As a result, the steel frame of a building may have to be protected by materials with certain insulating characteristics, and the building may have to include a sprinkler system if it is to meet the building code requirements of the locality in question.

# 1.2.3 Susceptibility to Buckling

As the length and slenderness of a compression member is increased, its danger of buckling increases. For most structures the use of steel columns is very economical because of their high strength-to-weight ratios. Occasionally, however, some additional steel is needed to stiffen them so they will not buckle. This tends to reduce their economy.

# 1.2.4 Fatigue

Another undesirable property of steel is that its strength may be reduced if it is subjected to a large number of stress reversals or even to a large number of variations of tensile stress. (Fatigue problems occur only when tension is involved.) The present practice is to reduce the estimated strengths of such members if it is anticipated that they will have more than a prescribed number of cycles of stress variation.

# 1.2.5 Brittle Fracture

Under certain conditions steel may lose its ductility, and brittle fracture may occur at places of stress concentration. Fatigue type loadings and very low temperatures aggravate the situation.

## 1.3 EARLY USES OF IRON AND STEEL

Although the first metal used by human beings was probably some type of copper alloy such as bronze (made with copper, tin, and perhaps some other additives), the most important metal developments throughout history have occurred in the manufacture and use of iron and its famous alloy called steel. Today, iron and steel make up nearly 95 percent of all the tonnage of metal produced in the world.<sup>1</sup>

Despite diligent efforts for many decades, archaeologists have been unable to discover when iron was first used. They did find an iron dagger and an iron bracelet in the Great Pyramid in Egypt, which they claim had been there undisturbed for at least 5,000 years. The use of iron has had a great influence on the course of civilization since the earliest times, and may very well continue to do so in the centuries ahead. Since the beginning of the Iron Age in about 1000 B.C., the progress of civilization in peace and war has been heavily dependent on what people have been able to make with iron. On many occasions its use has decidedly affected the outcome of military engagements. For instance, in 490 B.C. in Greece at the Battle of Marathon, the greatly outnumbered Athenians killed 6400 Persians and lost only 192 of their own men. Each of the victors wore 57 pounds of iron armor in the battle. (This was the battle from which the runner Pheidippides ran the approximately 25 miles to Athens and died while shouting news of the victory.) This victory supposedly saved Greek civilization for many years.

According to the classic theory concerning the first production of iron in the world, there was once a great forest fire on Mount Ida in Ancient Troy (now Turkey) near the Aegean Sea. The land surface supposedly had a rich content of iron and the heat of the fire is said to have produced a rather crude form of iron which could be hammered into various shapes. Many historians believe, however, that human beings first learned to use iron that fell to the earth in the form of meterorites. Frequently the iron in meteorites is combined with nickel to produce a harder metal. Perhaps early human beings were able to hammer and chip this material into crude tools and weapons.

Steel is defined as a combination of iron and a small amount of carbon, usually less than 1 percent. It also contains small percentages of some other elements. Although

<sup>&</sup>lt;sup>1</sup>American Iron and Steel Institute, *The Making of Steel* (Washington, D.C., not dated), p. 6.

some steel has been made for at least 2000–3000 years, there was really no economical production method available until the middle of the nineteenth century.

The first steel almost certainly was obtained when the other elements necessary for producing it were accidentally present when iron was heated. As the years went by, steel probably was made by heating iron in contact with charcoal. The surface of the iron absorbed some carbon from the charcoal, which was then hammered into the hot iron. Repeating this process several times resulted in a case-hardened exterior of steel. In this way the famous swords of Toledo and Damascus were produced.

The first large volume process for producing steel was named after Sir Henry Bessemer of England. He received an English patent for his process in 1855, but his efforts to obtain a U.S. patent for the process in 1856 were unsuccessful because it was shown that William Kelly of Eddyville, Kentucky, had made steel by the same process seven years before Bessemer applied for his English patent. Although Kelly was given the patent, the name Bessemer was used for the process.<sup>2</sup>

Kelly and Bessemer learned that a blast of air through molten iron burned out most of the impurities in the metal. Unfortunately, at the same time the blow eliminated some desirable elements such as carbon and manganese. It was later learned that these needed elements could be restored by adding spiegeleisen, which is an alloy of iron, carbon, and manganese. It was further learned that the addition of limestone in the converter resulted in the removal of the phosphorus and most of the sulfur.

Before the Bessemer process was developed, steel was an expensive alloy used primarily for making knives, forks, spoons, and certain types of cutting tools. The Bessemer process reduced production costs by at least 80 percent and allowed for the first time production of large quantities of steel.

The Bessemer converter was commonly used in the United States until after the turn of the century, but since that time it has been replaced with better methods, such as the open-hearth process and the basic oxygen process.

As a result of the Bessemer process, structural carbon steel could be produced in quantity by 1870, and by 1890, steel had become the principal structural metal used in the United States.

Today most of the structural steel shapes and plates produced in the United States are made by melting scrap steel. This scrap steel is obtained from junk cars, and scrapped structural shapes as well as from discarded refrigerators, motors, typewriters, bed springs, and other similar items. The molten steel is poured into molds which have approximately the final shapes of the members. The resulting sections, which are run through a series of rollers to squeeze them into their final shapes, have better surfaces and fewer internal or residual stresses than newly made steel.

The shapes may be further processed by cold rolling, by applying various coatings, and perhaps by the process of *annealing*. This is the process by which the steel is heated to an intermediate temperature range, held at that temperature for several hours, and then allowed to slowly cool to room temperature. It results in steel with less hardness and brittleness, but with greater ductility.

The term *cast iron* refers to materials with very low carbon content materials, while the very high carbon content materials are referred to as *wrought iron*. Steels fall

<sup>&</sup>lt;sup>2</sup>American Iron and Steel Institute, Steel 76 (Washington, D.C., 1976), pp. 5–11.