# TIMBER CONSTRUCTION MANUAL

THIRD EDITION 1985



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1985

# AMERICAN INSTITUTE OF TIMBER CONSTRUCTION

Englewood, Colorado

A WILEY-INTERSCIENCE PUBLICATION

JOHN WILEY & SONS

New York · Chichester · Brisbane · Toronto · Singapore

Published by John Wiley & Sons, Inc. 1966, 1974, 1985 by American Institute of Timber Construction.

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#### Library of Congress Cataloging in Publication Data:

Main entry under title:

Timber construction manual.

Includes index.

1. Building, Wooden-Handbooks, manuals, etc.

I. American Institute of Timber Construction.

TA666.T47 1985 694 85-7165 ISBN 0-471-82758-4

Printed in the United States of America

10 9 8 7 6 5 4 3 2 1

#### **PREFACE**

This Third Edition has been prepared to update the AITC Timber Construction Manual to reflect current timber design methods. Part I of the manual contains general design data and construction information. Part II contains information on loads and the design of structural elements and their fastenings. Part III contains reference information and AITC recommended standards and specifications for engineered timber construction.

The work of the preparation of the *Timber Construction Manual* was guided by the AITC Technical Advisory Committee and was carried out by AITC staff engineers and by engineers and technical representatives of AITC member firms.

Suggestions for the improvement of this manual will be welcomed and will receive consideration in the preparation of future editions.

The *Timber Construction Manual* has been adopted by the American Institute of Timber Construction as its official recommendation.

The American Institute of Timber Construction has developed this *Timber Construction Manual* for convenient reference by architects, engineers, contractors, teachers, and the laminating and fabricating industry, and all others having need for up-to-date technical data and recommendations on engineered timber construction.

While these data have been prepared in accordance with recognized engineering principles and are based on the most accurate and reliable technical data available, they should not be used or relied upon for any general or specific application without competent professional examination and verification of their accuracy, suitability, and applicability by a licensed professional engineer, designer, or architect. By the publication of this manual, AITC intends no representation or warranty, expressed or implied, that the information contained herein is suitable for any general or specific use or is free from infringement of any patent or copyright. Any user of this information assumes all risk and liability arising from such use.

#### PREFACE TO SECOND EDITION

The first edition of the AITC Timber Construction Manual was published in 1966. Changes in the wood products industry and technological advances and improvements in the structural timber fabricating industry have necessitated this revised edition of the Manual.

New lumber sizes and revisions in grading requirements for lumber and glued laminated timber are reflected in this second edition. Improved and refined design procedures are also incorporated.

The *Timber Construction Manual* was prepared by the AITC engineering staff with the guidance of the Institute's Technical Advisory Committee. The valuable assistance provided from many sources in developing technical data for the *Manual* is gratefully acknowledged.

#### PREFACE TO FIRST EDITION

In recent years, technical developments and the establishment of an engineered timber fabricating and laminating industry have had a profound effect on construction. Long clear spans of timber trusses, girders, arches, and decking are now commonplace. Engineered timber is widely used in such diversified construction as schools, churches, commercial buildings, industrial buildings, residences and farm buildings, highway and railway bridges, towers, theater screens, ships, and military and marine installations.

Modern practices combine engineering, quality control, and careful grading with the use of proper working stresses, dependable adhesives, and efficient mechanical fastenings to produce reliable construction. Laminating with strong, durable adhesives permits the manufacture of curved and variable shaped members and thus increases the versatility of timber construction.

The American Institute of Timber Construction is a nonprofit, technical, industrial association of manufacturers and fabricators who may design, plant-laminate, fabricate, assemble, and erect load-carrying sawn and glued timber framing and decking for roofs and other structural parts of schools, churches, commercial, industrial, and other buildings, and for other structures such as bridges, towers, and marine installations.

The American Institute of Timber Construction has developed this Timber Construction Manual for convenient reference by architects, engineers, contractors, teachers, the laminating and fabricating industry, and all others having a need for reliable, up-to-date technical data and recommendations on engineered timber construction. The information and the recommendations herein are based on the most reliable technical data available and reflect the commercial practices found to be most practical. Their application results in structurally sound construction.

The Manual has been arranged primarily for convenient use by designers, detailers, and fabricators of engineered timber construction. To avoid repetition, material which pertains to more than one area will be found in only one section. Suitable cross references are made in the other pertinent sections.

Information of an engineering textbook nature, such as derivations of formulae, is not included, since the purpose of the *Manual* is to present data for design and construction application by those familiar with engineering procedures.

Part I of the *Manual* contains design data and construction information. Part II contains AITC recommended standards and specifications which will aid the designer in preparing plans and specifications for engineered timber construction.

Material has been compiled from many sources. Where it has been possible

to identify the author of the material reproduced, it is used with the author's permission.

Every precaution has been taken to assure that all the data and information included are as accurate as possible. However, the Institute cannot assume responsibility for errors or omissions resulting from the use of this *Manual* in the preparation of plans or specifications. The Institute does not prepare engineering plans.

The work of the preparation of the *Timber Construction Manual* was guided by the AITC Technical Advisory Committee and was carried out by AITC staff engineers and by engineers and technical representatives of AITC member firms.

Suggestions for the improvement of this *Manual* will be welcomed and will receive consideration in the preparation of future editions.

The *Timber Construction Manual* has been adopted by the American Institute of Timber Construction as its official recommendation.

#### GENERAL NOMENCLATURE

The following abbreviations and symbols are in general use throughout this manual. Deviations from these notations are indicated where they occur.

#### ABBREVIATIONS

AASHTO American Association of State Highway and Trans-

portation Officials

AREA American Railway Engineers Association
AITC American Institute of Timber Construction
ANSI American National Standards Institute

APA American Plywood Association

ASCE American Society of Civil Engineers

ASTM American Society for Testing and Materials

AWPA American Wood-Preservers' Association

Btu British thermal unit
DL Dead load (psf)

EL Earthquake load (psf)

EMC Equilibrium moisture content (%)

FPL Forest Products Laboratory, U.S. Forest Service

ft, ft<sup>2</sup>, ft<sup>3</sup> feet, square feet, cubic feet

G Specific gravity
hr Hour

in., in.<sup>2</sup>, in.<sup>3</sup>, in.<sup>4</sup>... inches, square inches, cubic inches, inches to the fourth

power

in.-lb Inch-pounds

k Kip (one thousand pounds)

KD Kiln dried lb Pound

LL Live Load (psf)

MC Moisture content (%)

min Minimum

MSR Machine stress rated

#### xviii General Nomenclature

| NA | Neutral | axis |
|----|---------|------|
| NA | neutrai | axis |

NDS National Design Specification for Wood Construction

o.c. On centers

oF Degrees Fahrenheit

pcf Pounds per cubic foot

plf Pounds per lineal foot

psf Pounds per square foot

psi Pounds per square inch

SL Snow load (psf)
TL Total load (psf)

USDA United States Department of Agriculture

WL Wind Load (psf)

#### **SYMBOLS**

A

| $A_1$   | In fastener group analysis, cross-sectional area of main wood member(s) before boring and grooving (in. <sup>2</sup> )                 |
|---------|--|
| $A_2$   | In fastener group analysis, sum of cross-sectional areas of wood or metal side member(s) before boring or drilling (in. <sup>2</sup> ) |
| a       | Dimension of member (in.)  |
| а       | Distance to load for bracket columns (in.)   |
| $A_{c}$ | Area of concrete footing in pole design (ft <sup>2</sup> )   |
| $A_{s}$ | Area of steel member (in.2)  |
| b       | Breadth (width) of rectangular member (in.)  |
| b       | Smaller side of beam or column before exposure to fire   |

Area of cross section (in.<sup>2</sup>)

(in.)
b Width of column flange (in.)

C Compressive force (lb)

C Pole circumference at point of maximum moment (in.)

C Thermal conductance (Btu/hr ft² °F)

Distance from neutral axis to outer surface of beam (in.)

Curvature factor

 $C_{co}$  Seasoning conditioning modification factor for poles

 $C_{cs}$  Critical section modification factor for poles

 $C_{
m D}$  Duration-of-load factor  $C_{
m d}$  Depth-of-embedment factor

Constant for tapered beam deflection

C<sub>e</sub> Fastener edge distance factor

| $C_{\mathbf{f}}$              | Form factor   |
|-------------------------------|---|
| $C_{ m F}$                    | Size factor   |
| $C_{g}$                       | Group action factor   |
| $C_{ m I}$                    | Interaction stress factor   |
| $C_{\mathbf{k}}$              | For bending members, largest value of $C_s$ at which in-          |
| Ŭ <sub>k</sub>                | termediate-beam formula applies                                   |
| $C_{\text{L}}$                | Lateral stability-of-beams factor                                 |
| $C_{ m lb}$                   | Lag bolt modifying factor   |
| $C_{\mathbf{M}}$              | Moisture content factor   |
| $C_{\mathbf{M}}$              | Steel stress coefficient for bridge dowel design (psi)            |
| $C_{\mathbf{n}}$              | Fastener end distance factor                                      |
| $C_{ m P}$                    | Lateral stability-of-columns factor                               |
| $C_{ m p}$                    | Ponding magnification factor                                      |
| $C_{\mathbf{R}}^{\mathbf{r}}$ | Fire-retardant treatment factor                                   |
| $C_{\mathbf{R}}$              | Steel stress coefficient for bridge dowel design (psi)            |
| $C_{\rm r}$                   | Reduction factor for double-tapered curved beams                  |
| $C_{\rm s}$                   | Fastener spacing factor   |
| $C_{\mathrm{s}}$              | Slenderness factor for beam stability                             |
| $C_{ m SF}$                   | Modifier for safety factor for poles                              |
| $C_{ m st}$                   | Fastener steel side plate factor                                  |
| $C_{\mathrm{t}}$              | Temperature factor  |
| $C_{x}$                       | Spaced column fixity factor                                       |
| $C_{y}$                       | Factor for tapered beam deflection                                |
| D                             | Diameter (in.)  |
| d                             | Bridge dowel diameter (in.)                                       |
| d                             | Depth of rectangular member (in.)                                 |
| d                             | Least dimension of compression member (in.)                       |
| d                             | Larger side of beam or column before exposure to fire             |
|                               | (in.)   |
| $d_{ m b}$                    | Arch depth at base (in.)  |
| $d_{ m c}$                    | Depth of cross section at centerline (in.)                        |
| $d_{ m cb}$                   | Approximate centerline depth for double-tapered                   |
|                               | curved beams (in.)  |
| $d_{ m crt}$                  | Minimum centerline depth due to radial tension for                |
| d                             | double-tapered curved beams (in.)                                 |
| $d_{ m eb}$                   | Factor for calculating depth of double-tapered curved beams (in.) |
| $d_{ m eff}$                  | Approximate effective centerline deflection for double-           |
| ···CII                        | tapered curved beams (in.)  |
| $D_{ m H}$                    | Diameter of hole for pole design (ft)                             |
|                               | - 4 1   |

| хх                               | General Nomenclature |   |
|----------------------------------|----------------------|---|
| $d_{t}$                          |                      | Depth of tangent point (in.)  |
| $\boldsymbol{E}$                 |                      | Modulus of elasticity (psi)   |
| e                                |                      | Eccentricity of load (in.)  |
| f                                |                      | Dimensionless factor from Figure 1.2  |
| $F_{ m b}$                       |                      | Design value in bending (psi)   |
| $f_{b}$                          |                      | Bending stress (psi)  |
| $F_{\rm c}$                      |                      | Design value in compression parallel to grain (psi)                             |
| $f_{ m c}$                       |                      | Compression parallel to grain stress (psi)                                      |
| $F_{c\perp}$                     |                      | Design value in compression perpendicular to grain (psi)                        |
| $f_{\mathrm{c}oldsymbol{\perp}}$ |                      | Compression perpendicular to grain stress (psi)                                 |
| $f_{cr}$                         |                      | Ultimate column buckling strength (psi)   |
| $F_{\mathbf{g}}$                 |                      | Design value for end grain in bearing (psi)                                     |
| $f_0$                            |                      | Reference stress for double-tapered curved beams (psi)                          |
| $f_{f r}$                        |                      | Radial stress (psi)   |
| $F_{ m rt}$                      |                      | Design value in radial tension (psi)  |
| $f_{rt}$                         |                      | Radial tension stress (psi)   |
| $f_{ m s}$                       |                      | Torsional stress (psi)  |
| $F_{t}$                          |                      | Design value in tension parallel to grain (psi)                                 |
| $f_{\mathfrak{t}}$               |                      | Tension parallel to grain stress (psi)  |
| $F_{ m v}$                       |                      | Design value in horizontal shear (psi)  |
| $f_{\mathbf{v}}$                 |                      | Horizontal shear stress (psi)   |
| $\boldsymbol{G}$                 |                      | Shear modulus (modulus of rigidity) (psi)                                       |
| h                                |                      | Height of crown of arch (ft)  |
| $h_{\mathrm{a}}$                 |                      | Height of apex for double-tapered curved beams (in.)                            |
| $h_{\mathrm{s}}$                 |                      | Height of soffit at midspan for double-tapered curved beams (in.)               |
| I                                |                      | Initial moisture content (below 30%) (%)  |
| I                                |                      | Moment of inertia (in.4)  |
| $I_{ m K}/I_{ m G}$              | <b>!</b>             | Ratio of moment of inertia of knots to moment of inertia of gross cross section |
| $J_{\mathbf{x}}, J_{\mathbf{y}}$ | ,                    | Factor for column stability check   |
| K                                |                      | Constant for bridge deck design   |
| K                                |                      | Factor for intermediate columns   |
| K                                |                      | Bending stress factor for double-tapered curved beams                           |
| k                                |                      | Change in member thickness for arch deflection (%)                              |
| k                                |                      | Thermal conductivity (Btu in./hr ft <sup>2</sup> °F)                            |
| $K_{\rm e}$                      |                      | Effective buckling length factor  |
| $K_{\rm R}$                      |                      | Factor for round columns  |
| K <sub>r</sub>                   |                      | Radial stress factor  |
| -                                |                      |   |

| $K_1, K_2$                             | Coefficients for truss deflection                                  |
|--|--|
| L                                      | Span (ft)  |
| l                                      | Span length of beam or unsupported length of column (in.)          |
| Vd                                     | Span-to-depth ratio  |
| $L_{ m c}$                             | Length between tangent points for double-tapered curved beams (ft) |
| $L_{ m e}$                             | Effective length for shear (ft)                                    |
| $l_{\mathbf{e}}$                       | Unsupported column length (in.)                                    |
| $l_{ m e}$                             | Effective length of beam (in.)                                     |
| $l_{\mathrm{t}}$                       | Length of tapered leg for double-tapered curved beams (in.)        |
| $l_{ m u}$                             | Unsupported beam length (in.)                                      |
| M                                      | Moment capacity (in. lb)   |
| m                                      | Final moisture content (below 30) (%)                              |
| $M_{ m D}$                             | Moment capacities for dowel bridge design (in. lb)                 |
| $M_{ m s}$                             | Bending moment due to unit load (in. lb)                           |
| $M_{ m y}$                             | Total secondary moment for dowel bridge design (in. lb)            |
| N                                      | Fastener value for angle with direction of grain (lb)              |
| n                                      | Number of dowels for bridge deck design                            |
| P                                      | Axial load (lb)  |
| P                                      | Design wheel load for bridge design (lb)                           |
| þ                                      | Allowable passive soil pressure for poles (psf)                    |
| þ                                      | Fastener value for load acting parallel to grain (lb)              |
| Q                                      | Fastener value for load acting perpendicular to grain (lb)         |
| Q                                      | Statical moment of area (in.3)                                     |
| R                                      | Radius of curvature of inside face of lamination (in.)             |
| r                                      | Radius of gyration (in.)   |
| $R_{ m D}$                             | Shear capacities for dowel bridge design (lb)                      |
| $R_{H}$                                | Horizontal reaction (lb)   |
| $R_{ m m}$                             | Radius of curvature of centerline of curved member (in.)           |
| $R_{\mathrm{T}}, R_{1}, R_{2}, \ldots$ | Thermal resistance (hr ft <sup>2</sup> °F/Btu)                     |
| $R_{ m v}$                             | Vertical reaction (lb)   |
| $R_{y}$                                | Total secondary shear for dowel bridge design (lb)                 |
| S                                      | Section modulus (in.3)   |
| s                                      | Effective bridge deck span (in.)                                   |
| s                                      | Length of arch segment (in.)                                       |

Ω

| $S_{\mathbf{B}}$     | Allowable soil-bearing capacity for poles (psf)               |
|----------------------|---|
| $S_{ m m}$           | Shrinkage from initial moisture condition to final mois-      |
|                      | ture content $m(\%)$  |
| $S_{\rm n}$          | Section modulus times size factor (in. <sup>3</sup> )         |
| $S_0$                | Total shrinkage from Table 2.3 (%)                            |
| $S_0, S_1, S_3, S_4$ | Allowable lateral soil-bearing pressure for poles (psf)       |
| T                    | Applied torque (in. lb)                                       |
| T                    | Tensile force (lb)  |
| t                    | Bridge deck thickness (in.)                                   |
| t                    | Fire resistance rating (min)                                  |
| t                    | Thickness of column flange (in.)                              |
| t                    | Thickness of lamination (in.)                                 |
| U                    | Overall heat transfer coefficient (Btu/hr ft <sup>2</sup> °F) |
| u                    | Force in truss member caused by unit load (lb)                |
| V                    | Vertical shear force (lb)                                     |
| W                    | Total uniform load (lb)                                       |
| $\boldsymbol{w}$     | Uniform load (pounds per unit length)                         |
| W'                   | Total load of 1 in. of water (lb/in.)                         |
| X                    | Distance (ft)   |
| x                    | Distance (in.)  |
| x                    | Horizontal location (ft)                                      |
| y                    | Vertical location (ft)  |
| y                    | Wall height of arch (ft)                                      |
| α                    | Angle measure (degrees)                                       |
| $lpha_{ m r}$        | Radial coefficient of thermal expansion                       |
| $\alpha_{t}$         | Tangental coefficient of thermal expansion                    |
| $\Delta_{ m H}$      | Horizontal movement (in.)                                     |
| Δ                    | Deflection (in.)  |
| $\Delta_{c}$         | Centerline deflection (in.)                                   |
| $\theta$             | Angle measure (degrees)                                       |
| $\pi$                | Pi  |
| $\sigma_{ m PL}$     | Proportional limit stress for bridge dowel design (psi)       |
| φ                    | Angle measure (degrees)                                       |
|                      |   |

Coefficient of variation

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## Part I

### **GENERAL**