Introduction to SOIL MECHANICS









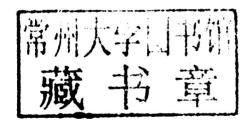
Béla Bodó & Colin Jones

WILEY Blackwell



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This edition first published 2013 © 2013 by John Wiley & Sons, Ltd

Registered Office

John Wiley & Sons, Ltd, The Atrium, Southern Gate, Chichester, West Sussex, PO19 8SQ, United Kingdom.

Editorial Offices

9600 Garsington Road, Oxford, OX4 2DQ, United Kingdom.

The Atrium, Southern Gate, Chichester, West Sussex, PO19 8SQ, United Kingdom.

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Library of Congress Cataloging-in-Publication Data

Bodo, Bela, (Engineer)

Introduction to soil mechanics / Bela Bodo, Colin Jones.

pages cm

Includes bibliographical references and index.

ISBN 978-0-470-65943-4 (pbk.: alk. paper) - ISBN 978-1-118-55387-9 (emobi) -

ISBN 978-1-118-55388-6 (epub) - ISBN 978-1-118-55389-3 (epdf) 1. Soil mechanics. I. Title.

TA710.B617 2013

624.1'5136-dc23

2012040913

A catalogue record for this book is available from the British Library.

Wiley also publishes its books in a variety of electronic formats. Some content that appears in print may not be available in electronic books.

Cover image courtesy of Shuttlestock.com Cover design by Steve Thompson

Set in 9/11.5pt Interstate-Light by SPi Publisher Services, Pondicherry, India Printed and bound in Malaysia by Vivar Printing Sdn Bhd

Introduction to Soil Mechanics

About the companion website

This book's companion website is at www.wiley.com/go/bodo/soilmechanics and offers invaluable resources for students and lecturers:

- · Supplementary problems
- Solutions to supplementary problems



www.wiley.com/go/bodo/soilmechanics

Preface

This book is intended to introduce the subject to students studying for BTEC Higher National Certificate/Diploma in Civil Engineering and Building Studies or for a Degree in Civil Engineering. It should also be practical reference to Architects, Geologists, Structural and Geotechnical Technicians.

The primary aim is to provide a clear understanding of the basic concepts of Soil Mechanics. We endeavoured to avoid the temptation of over-elaboration by providing excessively detailed text, unnecessary at this early stage of technical studies.

The purpose of this publication is threefold:

- 1. To introduce the student to the basics of soil mechanics.
- 2. To facilitate further advanced study.
- 3. To provide reference Information.

In order to satisfy the above requirements, the concepts of the subject are defined concisely, aided by diagrams, charts, graphs, tables and worked examples as necessary.

The text may appear to be excessively analytical at first sight, but all formulas are derived in terms of basic mathematics, except for a few requiring complicated theory, for those interested in working from first principles. They can be applied however, without reference to the derivation. The expressions are numbered and referred to throughout the text.

There are numerous worked examples on each topic as well as supplementary problems. All examples and problems are solved, many of them interrelated so that solutions can be compared and verified by means of several methods.

Some soil testing procedures are outlined only, as there are a number of excellent, detailed, specialized books and laboratory manuals available to cover this part of the subject.

There is some emphasis on the units employed and on the difference between mass and weight. This subject is discussed in Appendix A.

Béla Bodó and Colin Jones

Dedication

"I dedicate this book to my late wife Dorie." Béla Bodó

Acknowledgments

We wish to express our appreciation to Mr. Norman Seward, Senior Lecturer in Civil Engineering at the University of Wales College, Newport for his technical advice as to the presentation of the subject.

We are also grateful to Mr. Gregory Williams for his help in the production of this book. We would like to thank ELE International for their support in providing product images.

List of Symbols

CBR	California bearing ratio
C_{r}	Relative compaction
D_r	Relative Density
e	Voids ratio
G_{s}	Specific gravity
k	CBR Load-ring factor
M	Total Mass of sample
m	Moisture (water) content
m_{\circ}	Optimum moisture content
$M_{\rm s}$	Mass of solids
$M_{\rm w}$	Mass of water
n	Porosity
P	CBR applied force
P _a Q	Percentage of air voids
Q	CBR Load gauge reading
S	Degree of saturation
V	Total volume of sample
S	Volume of air
V_{c}	Volume of calibrating cylinder
V_{s}	Volume of solids
$V_{_{\vee}}$	Volume of voids
$V_{\rm w}$	Volume of water
W	Total weight of sample
W_{s}	Weight of solids
$W_{\rm w}$	Weight of water
δ	CBR Penetration distance (delta)
γ	Bulk weight density (Gamma)
γ'	Submerged weight density
$\gamma_{\rm d}$	Dry Weight density
γ_{d}	Dry Unit weight to be achieved by compaction
$\gamma_{\rm s}$	Weight density of solids
γ_{sat}	Saturated weight density
ρ	Bulk mass density
$ ho_{\scriptscriptstyle d}$	Dry mass density
$ ho_{sat}$	Saturated mass density
ho'	Submerged mass density
$ ho_{s}$	Mass density of solids

- $C_{\rm d}$ Correction for dispersing agent
- C_m Meniscus correction
- D Equivalent particle diameter
- D₁₀ Effective size of a particle
- f Specific Volume change
- H Height from the top of the bulb to surface
- $h_{\scriptscriptstyle h}$ Length of bulb
- Height of centre of bulb to surface
- LI Liquidity index
- LL Liquid limit
- M_0 Mass passing the n^{th} sieve
- M_{\star} Mass retained on the n^{th} sieve
- $m_{\scriptscriptstyle au}$ Temperature correction
- N Number of blows
- PI Plasticity index
- PL Plastic limit
- P_n Percentage of soil passing the n^{th} sieve
- R Mixing ratio
- R'_{h} Recorded hydrometer reading
- R_h Corrected hydrometer reading
- RI Relative consistence index
- SL Shrinkage limit
- T Temperature
- t Time
- U Uniformity coefficient
- u Velocity of sedimentation
- V_b Volume of hydrometer bulb
- V Volume of over-dried specimen
- ≈ Volume at SL
- x Magnitude of linear shrinkage or swelling
- Z Saturation limit
- η Dynamic viscosity $\langle eta \rangle$

- A Cross-sectional area of specimen
- a Cross-sectional area of standpipe
- A Cross-sectional area of solids in specimen
- A Cross-sectional area of voids in specimen
- EPL Equipotential line
- FL Flow Line
- F. Factor of safety
- GL Ground level
- GWL Groundwater level (Water Table)

h	Head	lacc
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- H_{τ} Total head at x
- H, Head loss to point x
- h_{x} Pressure head at x
- Hydraulic gradient
- Average hydraulic gradient
- Critical hydraulic gradient
- Exit gradient
- Coefficient of permeability
- L Length of flow path
- N Number of squares (head drops)
- N, Number of flow channels
- N, Number of head drops to point x
- P Hydrostatic force
- Q Flowrate
- Quantity of flow in time (t) q
- R Radius of influence
- Radius to observation well r
- Radius of central well
- Š Seepage force
- Seepage pore pressure at x u,
- Head Loss between equipotential line Δh
- Discharge velocity υ
- Seepage velocity v.

- 1 Influence factor
- n Number of elements on the Newmark chart
- Q Concentrated point load
- Uniformly distributed load (UDL) q
- Radius r
- Z Depth
- σ Horizontal pressure
- σ_{ij} Vertical pressure
- Shear stress τ

- dh Total deformation of specimen of thickness h
- h_A Artesian pressure head
- h_c Capillary head
- h Seepage pressure head
- Critical hydraulic gradient
- $m_{\scriptscriptstyle E}$ Equilibrium moisture content
- Optimum moisture content m_o
- pF Soil suction index
- PI Plasticity index
- Degree of saturation S,

- S. Soil suction
- T Surface tension
- u Pore pressure
- $u_{\rm cs}$ Pore pressure in the capillary fringe
- u_h Static pore pressure at depth h
- u. Seepage pore pressure
- z_c Critical depth for piping
- Δu Small change in u
- $\Delta \gamma$ Change in unit weight
- $\Delta\sigma$ Small change in σ
- $\Delta\sigma'$ Small change in σ'
- δ Deformation of specimen at time t
- σ Total pressure
- σ' Effective pressure
- $\sigma_{\!\scriptscriptstyle \Delta}$ Artesian pressure

- A Pore pressure coefficient
- A Pore pressure coefficient
- B Pore pressure coefficient
- c Cohesion
- c Undrained shear strength
- CD Consolidated-drained test
- CU Consolidated-undrained test
- ESP Effective stress path
- NCC Normally consolidated clay
- n Proving ring constant
- OCC Over consolidated clay
- p&q Stress path coordinates
- p,&q. Stress path coordinates at failure
- QU Quick-undrained test
- r_x Force dial reading at x
- TSP Total stress path
- UU Unconsolidated-undrained test
- x Strain gauge reading
- $\Delta u_{\rm d}$ Change in pore pressure due to $\Delta \sigma_{\rm d}$
- Δu_c Change in pore pressure due to $\Delta \sigma_c$
- $\Delta\sigma_{_{\!c}}$ Change in cell pressure
- $\Delta\sigma_{_{\! d}}$ Change in the deviator stress
- ε Strain at x
- ϕ Angle of friction
- σ_{0} Normal pressure
- σ Deviator stress at x
- τ Shear stress
- τ_{i} Shear stress at failure
- $\tau_{_{D}}$ Shear stress on a plain
- au_m Maximum shear stress

- Area indicating completed consolidation A
- Area under an isochrone A_{t}
- Coefficient of compressibility a
- Coefficient of Secondary settlement () to consolidation
- Compression index
- C_{u} Coefficient of consolidation
- D, Dial reading at stage x
- dH. Initial settlement
- E Modulus of elasticity
- Initial voids ratio eo
- Final voids ratio e,
- Voids ratio after swelling e_s
- Voids ratio at stage x
- H Layer thickness
- H_{\circ} Flow path
- h, Height of specimen at stage x
- Influence factor
- Coefficient of permeability k
- Coefficient of volume change m.
- OCR Overconsolidation ratio
- Bearing pressure q
- T Time factor
- Time t
- U Average degree of consolidation
- U, Degree of consolidation
- U Pore pressure at time t
- u_{o} Initial pore pressure
- Long-term consolidation settlement ΔH
- $\Delta\sigma'$ Effective consolidating pressure
- δ Depth factor (Delta)
- Poisson's ratio (My)
- σ' Effective pressure at stage x

- Unconfined compression strength C_{u}
- Adhesion between soil and wall c_{w}
- Eccentricity е
- Factor of safety in terms of friction angle
- Maximum compressive stress f_{max} Minimum compressive stress
- f_{min} F_s H Factor of safety
- Height of wall
- H_{0} Height of unsupported clay
- Coefficient of lateral pressure

K_{\circ}	Coefficient of earth pressure at rest
Ka	Coefficient of active earth pressure
K,	Coefficient of earth pressure at failure
K_{p}	Coefficient of passive earth pressure
L,	Length of slip surface
M_{max}	Maximum bending moment
$M_{\rm o}$	Overturning moment
$M_{\rm R}$	Resisting moment
P_{a}	Active force
P_{p}	Passive force
$P_{\rm w}$	Force of water in tension crack
R	Force on wedge
T	Tension force in tie rod
Z_{c}	Pile penetration
δ^{z_0}	Depth of tension crack
	Angle of wall friction
ϕ'_{m}	Mobilised friction
μ	Coefficient of friction
$\sigma_{_{\! a}}$	Active earth pressure
$\sigma_{_{\!\scriptscriptstyle c}}$	Cell pressure in triaxial test
$\sigma_{_{\! d}}$	Deviator stress in triaxial test
$\sigma_{_{\!\scriptscriptstyle \mathrm{p}}}$	Passive earth pressure
$\sigma_{_a}'$	Effective active earth pressure
$\sigma'_{\mathfrak{p}}$ $\sigma'_{\mathfrak{p}}$	Effective passive earth pressure
$\bar{\sigma}$	Average pressure
τ_{ϵ}	Shear stress at failure

\overline{C}_{ij}	Average undrained shear strength
$A_{\rm e}$	End bearing area
A_{s}	Surface area of pile
В	Width of footing
С	Cohesion
$F_{\rm o}$	Overall factor of safety
F _o F _s K _s	Factor of safety
Ks	Average coefficient of earth pressure
1	Length of pile
N	Number of SPT blows
n	Number of piles
N'	Corrected value of N
N_c	
N _o >	Bearing capacity factors
N,	
P	Failure load on pile
Q	Design working load
Q_{a}	Allowable carrying capacity of pile
Q_{ag}	Allowable carrying capacity of pile group

- Q_{ρ} End bearing resistance
- Q_{t} Negative skin friction
- Q_{c} Shaft resistance
- Ultimate carrying capacity of pile $Q_{..}$
- $Q_{\rm ug}$ Ultimate carrying capacity of pile group
- Net ultimate bearing capacity q_n
- Safe bearing capacity q_s
- Safe net bearing capacity $q_{\rm sn}$
- Ultimate bearing capacity q_{\parallel}
- SPT Standard penetration test
- $W_{_{\rm P}}$ Weight of pile
- Adhesion factor (Alpha) α
- δ Angle of friction between soil and pile (Delta)
- Efficiency of pile group (Eta) η
- Angle of friction φ
- σ Safe bearing pressure of footing
- σ_{n} Net bearing pressure of footing
- $\bar{\sigma}_{\rm o}'$ Average effective overburden pressure
- σ'_{0} Effective overburden pressure

- Shear strength
- Friction force
- Factor of safety with respect to cohesion
- Factor of safety
- F , L Factor of safety with respect to friction
- Length of slip surface
- $M_{\scriptscriptstyle \mathrm{D}}$ Disturbing moment
- M_{R} Resisting moment
- N Normal (or radial) component of W
- $N_{\rm c}$ Stability number
- Radius of slip circle R
- Pore pressure ratio $r_{\rm u}$
- S Shear force
- T Tangential component of W
- W Weight

Chapter 11

The comprehensive list of symbols for EC7 is given in Eurocode 7. Geotechnical design Part 1: General rule. Only some of the symbols, applied in this book, are reproduced here:

- E_{d} Design value of the effect of actions
- $E_{\scriptscriptstyle
 m dst;d}$ Design value of the effect of destabilizing action
- $E_{\rm stb:d}$ Design value of the effect of stabilizing action
- $F_{\rm d}$ Design value of an action
- F_{rep} F_s Representative value of an action
- Factor of safety

G _{dst;d}	Design value of destabilising permanent action
$G_{\text{stb;d}}^{\text{dst,d}}$	Design value of stabilising permanent action
$Q_{dst;d}$	Design value of destabilising variable action
R_{α}	Design value of resistance action
S _{dst;d}	Design value of destabilising seepage force
I d	Design value of total shear resistance
U _{dst;d}	Design value of destabilising pore water pressure
$V_{\rm dst;d}$	Design value of destabilising vertical action
X_{d}	Design value of a material property
X_{k}	Characteristics value of a material property
$\gamma_{\scriptscriptstyle G}$	Partial factor for a permanent action
$\gamma_{G;dist}$	Partial factor for a destabilising action
$\gamma_{G:stb}$	Partial factor for a stabilising action
$\gamma_{\rm m}$	Partial factor for soil parameters (material property)
γ.	Partial factor for a variable action

Partial factor for sliding resistance

 $\gamma_{\rm R;h}$

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