# THEORY OF ELASTICITY AND PLASTICITY

YANG GUITONG Ph.D. Professor of Mechanics

# THEORY OF ELASTICITY AND PLASTICITY

YANG GUITONG Ph.D. Professor of Mechanics

INSTITUTE OF APPLIED MECHANICS
TAIYUAN UNIVERSITY OF TECHNOLOGY

#### 图书在版编目(CIP)数据

弹塑性理论:英文/杨桂通编著.一北京:中国建材工业出版社,2004.12

ISBN 7-80159-698-6

I.弹... II.杨... III.①弹性力学-英文②塑性力学-英文 IV.034

#### 中国版本图书馆 CIP 数据核字(2004)第 119754 号

责任编辑 阎 競封面设计 意博视觉

# 弹塑性理论

杨桂通 编著

# 出版发行:中间建材工业出版社

地 址:北京市西城区车公庄大街6号

邮 编:100044

经 销:全国各地新华书店

印 刷:北京鑫正大印刷有限公司

开 本:787mm×960mm 1/16

印 张:18.5

字 数:352 千字

版 次:2005年2月第一版

印 次:2005年2月第一次

定 价:49.00元

网上书店:www.ecool100.com

本书如出现印装质量问题,由我社发行部负责调换。联系电话:(010)88386904

# **Preface**

This book is based on my previous version in Chinese published 20 years ago. It follows the same style as the original book, i.e., elasticity and plasticity are combined in one textbook. To organize the materials in this way reflects the actual process in the real world. It is a continuous process that when a solid body or structure subjects to a gradually increased loading, it may cause the deformation from elastic state to plastic state, and then finally lose its designed functions. To teach students those two theories together may help to compare those two processes, for the understanding of the entire deformation process, and therefore help students to build solid knowledge and systematic concepts about elasticity and plasticity. It will also benefit the research of the theory as well as engineering designs.

Comparing with the Chinese version of *Theory of Elasticity and Plasticity*, a lot of changes have been made in this version. Almost every chapter has added new contents. Theory of plates and shells has been removed due to a recent trend of teaching this topic in a separate course. Meanwhile, dynamic problems have been included as a new chapter. The major additions reflect the developments and extensions of interest and practical applicability that have occurred since the appearance of the Chinese version in 1979.

This book is still easy to read and understand. Several efforts have been made to achieve this result by introducing the most recent reference articles, using less complicated formulas and avoiding abstract concepts in mathematics. It should help the students to overcome the difficulties while firstly entering this field, and it should also help them to focus on the most important concepts rather than immerse in complicated mathematics formulas.

I wish that this book would serve as a practical textbook for senior engineering students or graduates studies. It should also be used as a convenient reference book for engineers to solve the problems in their daily design work.

In the new period of development of science and technology, more and more engineering students will face new challenges when they join work force in the next few years. To adapt English textbooks will definitely help them more efficiently reading and writing technical articles in English.

I wish to express my gratitude to Professor Xu Bingye and Professor Yang Huizhu for their frequent help and valuable suggestions. I also acknowledge the assistance of my students and colleagues of the Institute of Applied Mechanics of Taiyuan University of Technology Dr. Chen Weiyi, Dr. Shu Xuefeng, Dr. Ma Hongwei, Dr. Zhang Nianmei and Professor Cai Zhongmin, for their support and help in completing this work.

杨桂通 Yang Guitong Jan. 27, 2002

# **CONTENTS**

1	Introduction · · · · · · · · · · · · · · · · · · ·		
	1.1 Elasticity and Plasticity		
1.2 Basic Hypothesis ·····			
	1.3 Historical Remarks		
2	Stress		
	2.1 Force and Stress		
	2.2 Stress Tensor and Stress Deviation Tensor		
	2.3 Octahedral Stresses and Maximum Shear Stress		
	2.4 Two-dimensional Stress State		
	2.4.1 Plane Stress Problem · · · · 12		
	2.4.2 Stress Boundary Condition		
	2.4.3 Stress Equilibrium Equation · · · · · 14		
	Problems		
3	Strain · · · · · 20		
	3.1 Deformation and Strain Tensor · · · · 20		
	3.2 Principal Strain and Octahedral Strain 25		
3.3 Strain Deviator Tensor ·····			
	3.4 Equations of Strain Compatibility		
	Problems		
4	Constitutive Relations		
	4.1 Generalized Hooke's Law ···· 3		
	4.2 Yield Conditions		
	4.2.1 The Tresca Yield Condition		
	4.2.2 The Mises Yield Condition		
	4.2.3 The Mohr-Coulomb Condition 4.		
	4.2.4 The Drucker-Prager Condition		
	4.3 Strain Hardening Models 4.		

	4.4 Drucker's Postulate and Il'yushin's Postulate			
	4.5	Plastic	Constitutive Relations	51
		4.5.1	The Concept of Elastic Potential and Plastic Potential	
		4.5.2	Incremental Strain Theory	
			The Total Strain Theory	
5	Form		and Solution of the Elastoplastic Problems	
	5.1		Equations	
	5.2 Methods of Solution for Elastoplastic Problems			
		5.2.1	Displacement Method of Elasticity	
		5.2.2	Stress Method of Elasticity	
		5.2.3	The Uniqueness of Solutions in Elasticity	
		5.2.4	Saint-Venant's Principle and Principle of Superposition	
			Methods of Solution in Plasticity	
	2 7 7 7			
6	Elasti		Problems ····	
	6.1		Equations	
	6.2		Function. Inverse and Semi-Inverse Method	
	6.3		ng of Elastic Beams	
	6.4		Beam Problem Solution by Trigonometric Series	
	6.5		Equations in Polar Coordinates	
	6.6		ng of Curved Beams ·····	
		6.6.1	Pure Bending of a Circular Beam	
		6.6.2	Bending of a Cantilever Circular Arc Beam	94
	6.7		e Plate with a Small Hole by Uniform Tension,	
	Stress Concentration			
	6.8		emi-Infinite Elastic Plane Problems	
		6.8.1	Elastic Wedge Loaded at the Vertex	
		6.8.2	Elastic Semi-Infinite Plane Problems ·····	
7	-		c-Plastic Problems · · · · · · · · · · · · · · · · · · ·	107
	7.1		c-Plastic Bending of Beams	107
	7.2		e-Plastic Solution of the Thick-Walled Tube	109
	7.3		e-Plastic Solution of a Rotating Solid Disc	115
	7.4	Elastic	e-Plastic Solution of a Rotating Cylinder	119

	Prob	olems ····		121
8	Solution of Elastic Plane Problems			
	by Means of Complex Variables			122
	8.1	Some I	Fundamental Relations of the	
		Theory	of Complex Variables	122
		8.1.1	Complex Variables and Complex Functions	122
		8.1.2	Goursat's Formula and Kolosoff-Muskhilishvili Functions	124
	8.2	The E	xpression of Displacements and Stresses	
		in Ter	ms of Analytic Functions	125
	8.3	Bounda	ary Conditions	126
	8.4	Condit	ions of Single-Valued Stresses and Displacements	128
	8.5	Confor	mal Mapping and Its Application	132
	8.6	Solution	n for an Infinite Plate with a Circular Hole	135
	8.7		e Region Bounded by an Ellipse	
9	Plasti	c Plane	Strain and Slip Line Field Theory	
	9.1		Equations	
	9.2	-	ne Field Theory	
	9.3	Basic I	Boundary Value Problems	
		9.3.1	The First Boundary Value Problem	
		9.3.2	The Second Boundary Value Problem	151
		9.3.3	The Third(Mixed) Boundary Value Problem	
	9.4		ant Properties of Slip Lines	
	9.5	Bounda	ary Conditions and Simple Stress States	
		9.5.1	Boundary Conditions	
		9.5.2	Simple Stress States	
	9.6	Displac	ement Velocity Equations	
	9.7	Some A	Application Examples	
		9.7.1	Indentation by a Die	
		9.7.2	Bending of a Short Cantilever Beam	167
		9.7.3	Full Plastic State of a Thick-Walled Cylinder	
10	Elas		ic Torsion of Prismatic Bars	
	10.1		ral Concepts and Elastic Torsion	
	10.2	2 Elasti	c Torsion of Rectangular Bars	177

	10.3	Plastic Torsion ·····	181	
	10.4	Methods of Analogy		
	10	0.4.1 Membrane Analogy		
		0.4.2 Sand-Hill Analogy ·····		
		0.4.3 Membrane-Roof Analogy		
		ms ·····		
11	Energy	y Principles in Elasticity		
	11.1	Energy Principles ·····		
	11.2	The Principle of Virtual Displacements	191	
	11.3	The Complementary Energy Concept and		
		the Principle of Virtual Stresses		
	11.4	Principle of Minimum Potential Energy		
	11.5	Principle of Minimum Complementary Energy		
	11.6	Generalized Variational Principles	201	
	11.7	Approximate Methods Based on the Variational Principles		
	1	11.7.1 The Ritz Method ·····		
	1	1.7.2 The Galerkin Method ·····		
	1	1.7.3 The Method of Kantorovich ·····		
	1	1.7.4 The Finite Element Method ······		
	11.8	Illustrative Examples ·····		
		ms		
12	Extre	num and Variational Principles in Plasticity		
	12.1	Principle of Virtual Velocity ·····	221	
	12.2	Extremum Principle of Stress Rate for a		
		Elastic-Plastic Materials	224	
	12.3	Extremum Principle of Strain Rate for		
		Elastic-Plastic Materials		
	12.4	Extremum Principle of Rigid-Plastic Materials		
	12.5	Principle of Maximum Plastic Work		
	12.6	Variational Principles for Deformation Theory of Plasticity		
	12.7	Theorems of Limit Analysis		
	12.8	Shakedown Theorems		
	12.9	Illustrative Examples ·····		
	Problems			

13	Dynamic Pro	blems	244
	13.1 Basic	Concepts	244
	13.1.1	Wave and Vibration	
		(Traveling Wave and Standing Wave)	244
	13.1.2	Reflection of Waves	248
	13.2 Dynan	nic Constitutive Relations	249
	13.2.1	Dynamic Properties of Material	249
	13.2.2	Dynamic Stress-Strain Relation	250
	13.3 Genera	al Dynamic Principles	252
	13.3.1	Hamilton Principle and Generalized	
		Hamilton Principle ·····	252
	13.3.2	Displacement Bound Theorems	253
	13.4 Dynan	nic Response of Elastic-Plastic Beams	255
	13.4.1	Characteristics of Rigid-Plastic Beam	261
	13.4.2	Illustrative Examples ·····	263
	13.5 Dynar	mic Response of Elastic and	
	Rigid-	Plastic Circular Plates	268
	13.5.1	Free Vibration of Elastic Circular Plate	269
	13.5.2	Dynamic Response of Rigid-Plastic Circular Plate	270
	13.6 Elastic	c-Plastic Loading Waves	273
	13.6.1	Elastic Wave and Application of the	
		Method of Characteristics	
	13.6.2	Elastic-Plastic Loading Wave	
	13.7 Unloa	ding Wave	277
	Problems ····		279
Sub	ject Index ····		284

# 1 Introduction

# 1.1 Elasticity and Plasticity

Essential properties of deformable bodies subjected to external force or other external action are elastic and plastic behaviors. As discussed in the discipline of **mechanics of materials**, that is, if the external forces producing deformation do not exceed a certain limit, that is so called **yield criteria**, the deformation disappears with the removal of the forces, and then we consider this property as elasticity. Otherwise, the deformation does not disappear after removal of the forces, and then we consider the property as plasticity. Another main difference between perfect elasticity and plasticity, in mathematical view, is a linear problem and a nonlinear problem, respectively.

The atom forces in the material internal structure determine the mechanism of these two kinds of deformations. In fact, the internal structure of solid materials is always stable, on the basis of balance forces between atoms in a solid. The suction force makes the atoms tend to close upon to each other, and the repulsion force makes the atoms maintain some reasonable distance. In normal cases, these two forces are in equilibrium state. Atomic structure will not be considered here. We will be interested in the macroscopical response only. When a solid body is

subjected to external loading, there are two different responses: elastic response and plastic response.

Elastic deformation is a simple case easy to be understood. Plastic deformation is a more complex case. Fig. 1.1 shows the typical curve for a simple tension specimen of metal. The initial elastic region generally appears as a straight line OA, where A

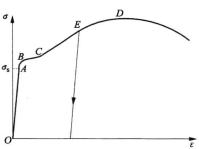


Fig. 1.1 Stress-strain diagram for an annealed cast-steel specimen

defines as the **limit of proportionality.** On further straining, the relation between stress and strain is no longer linear but the material is still elastic, and with release of the load, the specimen reverts to its original length. The maximum stress point B at which the load can be applied without causing any permanent deformation defines as the **elastic limit**. The point B is also called the **yield point**, for it marks the initiation of plastic or irreversible deformation. Usually, there is very little difference between the proportional limit, A, and the elastic limit, B. The behavior in the flat region BC is generally referred to as **plastic flow**. After C the material is exhibited strain hardening or also known as work hardening. Over some point D the material may be exhibited strain softening, as shown in Fig. 1.1.

Now, consider the unloading from some point E beyond the yield point. The behavior is as indicated in Fig. 1.1. That is, when the stress is reduced, the strain decreases along an almost elastic unloading line parallel to OA. So we say that the unloading obeys the elastic rule.

Fig. 1.2 is the typical graph of stresses versus relative elongation (compression) for four kinds of materials.

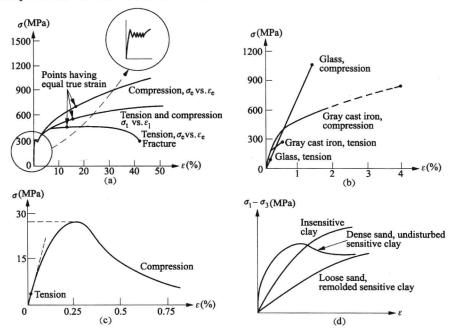


Fig. 1.2 Stress-strain diagrams: (a) Ductile metal, (b) Cast iron and glass, (c) Typical concrete or rock, (d) Soils, triaxial compression. (Experimental data are taken from reference[15].)

# 1.2 Basic Hypothesis

The subject of theory of elasticity and plasticity is concerned with the deformation and motion of elastic-plastic bodies or structures under the action of applied load or other disturbances. The general assumptions employed in the study of theory of elasticity and plasticity are the same as those used in the mechanics of continuous medium. Therefore, throughout this book, we have: (a) **continuum hypothesis**, we shall suppose that the macroscopic behavior of the solid bodies is the same as if they were perfectly continuous in structure; and physical quantities such as the mass and momentum associated with the matter contained within a given small volume will be regarded as being spread uniformly and without any caves, cracks and discontinuous. (b) **uniform hypothesis and isotropic hypothesis**, that are, the materials of a elastic-plastic body are homogeneous and uniformly distributed over its volume so that the smallest element cut from the body possesses the same specific physical properties as the body. The elastic properties are the same in all directions. (c) **small deformation hypothesis**, in this book, we discuss small deformation only.

# 1.3 Historical Remarks

Before the engineering design of structures, one must not only know the internal force field acting on the structural materials, but also know the material response. It means that we need give an analysis of the stresses, deformation and displacement of structural elements. Therefore we have to know the constitutive relation of the materials. Seeking some methods to solve these problems, many researchers have continually studied for over 2000 years.

The pioneering works of theory of elasticity and plasticity were given by Augustin Cauchy (1789—1857), Marie-Henri Navier (1785—1836), Leonard Euler (1707—1783), Simon Denis Poisson (1781—1840), Barre de Saint-Venant (1797—1886), Nikolai Ivanobich Mushihailishibili (1891—1976), Ludwig Prandtl (1875—1953), Thomas Young (1773—1829), Richard von Mises (1883—1953), and many others.

The general principles employed in the study of theory of elasticity and plasticity are the same as those used in studying the mechanics of continuous medium. Their basic formulations can be attributed primarily to the work of Euler and Cauchy. Euler first brought forward the general principles of linear and angular

momentum balance for continuous media upon which rest all continuum mechanics, including theory of elasticity and plasticity. Cauchy first gave the concept of the stress and strain at a point and also found the general differential equations of motion or equilibrium of a continuum in term of the stress. Cauchy's work on elasticity provided a detailed kinematical theory of strain and deformation. The extension of the mathematical theory to more general solids was first made by Navier in 1821 using special assumption concerning the molecular forces of elastic solids. Technical application began earliest in 1855, when Saint-Venant solved the problem of the twisting of prismatic bars and worked out detailed numerical results. Saint-Venant also took up the problem of plastic flow and developed two-dimensional governing equations which were subsequently generalized to three dimensions by M. Levy in 1871. In 1864 H. Tresca reported experiments to the French Academy, which suggested that the plastic yielding of a metal occured when the maximum shear stress reached to a critical value. After Tresca, in 1913 R. von Mises published his yield condition theory based on theory of distortional energy.

In the last century (1901—2000), the theory of elasticity and plasticity had been rapidly developed in theory and engineering practice. Many great contributors should be mentioned, such as B. G. Galerkin, G.R. Kirchhoff, S.P. Timoshenko, J. L. Lagrange, A. Nadai, A. A. Il'yushin, W. W. Sokolovsky, W. Prager, R. Hill, Kh. A. Rakhmatulin, G. I. Taylor, P. Perzyna, and many others.

In this period, especially in last 50 years, theory of elasticity and plasticity rapidly developed in China too. Qian Xuesen, Qian Weichang, Hu Haichang, Wang Ren, Huang Kezhi, Xu Bingye, Wu Jike, Huang Zhuping, Gao Yuchen, Wang Ziqiang, and many others developed the theory of elasticity and plasticity, specially in the engineering applications. In this period many valuable books about elasticity and plasticity on theoretical and engineering applications were published.



# 2 Stress

### 2.1 Force and Stress

There are two kinds of forces: body force and surface force. The force acting on each internal particle of body or structural members is so called **body force**. For example, the gravity force, inertia force, the electromagnetic force and mass force, etc. are all the body forces. The force acting on the surface of the body or structure is so called **surface force**. For example, the wind load, the fluid pressure, the contact force between two bodies, etc. are all the surface forces.

Now we discuss the magnitude and direction of body force and surface force of a body in a coordinate Oxyz. Let us take a volume element  $\Delta V$  at a point C, if the body force of  $\Delta V$  is  $\Delta F$ , then the mean density of body force in this volume element  $\Delta V$  is  $\Delta F/\Delta V$ . When  $\Delta V$  approaches to point C, the  $\Delta F/\Delta V$  will approach to a limit vector  $F_b$ 

$$\lim_{\Delta V \to 0} \frac{\Delta F}{\Delta V} = F_{\rm b} \tag{2.1}$$

Obviously, the direction of the body force vector  $F_b$  coincides with the limit direction of body force in  $\Delta V$ . The unit of the body force is N/m³. Suppose  $F_b$  is the mass force of unit mass at a neighborhood of point C, and let  $m = \rho dV$  is the mass of volume element dV,  $\rho$  is the density of mass,  $mF_b$  is the force acting on the mass of dV. Therefore the body force of unit volume is  $\rho F_b$ .

Similarly, we can define the surface force vector  $F_{\rm s}$ 

$$\lim_{\Delta S \to 0} \frac{\Delta F}{\Delta S} = F_s \tag{2.2}$$

and the surface force acting on surface element dS is  $F_s dS$ .

We now give the concept of stress and denote the vector of internal force  $\Delta p$  acting on the element of area  $\Delta S$ , cut out from the cross section C at any point P

(Fig. 2.1). The outward normal vector is represent by n. We observe that the

force acting on this elemental area, due to the action of material of the part B (which we throw aside) on the material of part A, can be reduced to a resultant  $\Delta p$ . If we continuously contract the elemental area  $\Delta S$ , the limiting value of the ratio  $\Delta p/\Delta S$  gives us the magnitude of the stress acting on the cross section C at any point P. The limit direction of the resultant  $\Delta p$  is the

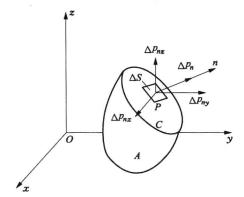


Fig. 2.1 Stress vector at a point P

direction of the stress. In the general case the direction of the stress is inclined to the area  $\Delta S$  on which it acts and we can resolve it into two components: a normal stress perpendicular to the area and a shearing stress acting in the plane of the area  $\Delta S$ . The stress vector  $\sigma$  is defined in terms of these quantities by the following equation

$$\sigma = \lim_{\Delta S \to 0} \frac{\Delta p}{\Delta S} \tag{2.3}$$

We now introduce the concept of stress tensor. To do this we first consider a small hexahedral element of material about some point P in the body. Let the faces of this element be parallel to the coordinate planes and on each of the six positive faces, whose outward normals in the positive directions, resolve the associated stress vectors into components along the coordinate directions as shown in

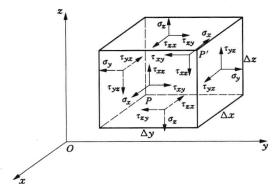


Fig. 2.2 Components of stress vector

Fig. 2.2. The notations used in this Figure in denoting the components of the stress vectors acting on each of the positive faces are as follows: The first subscript denotes the coordinate axis along which the outward normal of the considered face points and the second subscript denotes the direction in which the component acts. Thus for

example  $\sigma_{xy}$  denotes the stress component of the stress vector acting on the faces with normal along axis x, its direction is toward axis y. As a rule, the tension stress is considered as positive stress, a compression stress is considered as a negative stress. The components of stress tensor can be written by using a common rule, which  $\sigma$  represents a normal component of stress, and  $\tau$  represents shearing components of stress. From Fig. 2.2, it is easy to understand that when the small hexahedral approaches to a limit as 'infinitesimal', the stresses on the hexahedral represent the stress state at point P. Therefore the stress state at point P has all total 9 components, in which 3 normal stresses and 6 shear stresses (in fact, according to the **shear stress reciprocal theorem**, there are 3 shear stresses only).

## 2.2 Stress Tensor and Stress Deviation Tensor

The 9 components of stress may be represented by a second-order tensor, in which every row has 3 components of stress acting on one face at point P as follows

$$\sigma_x$$
  $\tau_{xy}$   $\tau_{xz}$ 
 $\tau_{yx}$   $\sigma_y$   $\tau_{yz}$ 
 $\tau_{zx}$   $\tau_{zy}$   $\sigma_z$ 

These 9 components of stress define a new physical quantity  $\Sigma$ , and it describes just the stress state at point P. When we make a coordinate transformation, every component of stress may change its quantities. But the new physical quantity  $\Sigma$  is unchanged at the same point P. As we know from mathematics, when coordinate transforms of elements obey a given coordinate transformation rule, then they determine a second-order tensor, as is so called **stress tensor**. Later we will see that the stress tensor is a symmetrical second-order tensor

$$\sigma_{ij} = \begin{bmatrix} \sigma_x & \tau_{xy} & \tau_{xz} \\ \tau_{yx} & \sigma_y & \tau_{yz} \\ \tau_{zx} & \tau_{zy} & \sigma_z \end{bmatrix}$$
 (2.4)

Here i, j (=1, 2, 3) associate with the axes x, y, z, and i represents rows and j represents columns, and the normal stress is represented by  $\sigma$ , the shear stress is represented by  $\tau$ . Therefore, it is clear that stress tensor perfectly determines the stress state at a given point.

Now we discuss the principal stresses. Let that the direction n at a point in a body is so oriented that the resultant stress vector  $p^n$ , associated with direction is