Anders C. Nilsson and Bilong Liu

# Vibro-Acoustics

/Volume II/



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Volume II

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#### PREFACE

The second volume of *Vibro-Acoustics* includes eight chapters. As in the first volume, each chapter ends with a number of problems. The solutions are given in a separate volume, which also contains a summary of some of the most important governing equations from the first two volumes.

A number of measurement results are presented in Volume II. Most of these measurements could not have been performed without the expert help of Fritiof Torstensson and Arne Jagenäs at Chalmers, Knut Ulvund at DNV and Kent Lindgren and Danilo Prilovic at KTH. I also gratefully acknowledge the pioneering work by Prof T.Kihlman, who introduced the field of vibro-acoustics in Scandinavia.

I would like to express my gratitude to Hector Valenzuela, Edoardo Piana and in particular to Benedetta Grassi for their untiring and very efficient work on preparing all the figures.

Any comments or questions on the text are most welcome. Contact us by email, address andersc.nilsson@gmail.com and liubl@mail.ioa.ac.cn.

Anders Nilsson Genova, Italy, March 2013

## CONTENTS

PREFA	CE			
Chapter 9 HAMILTON'S PRINCIPLE AND SOME OTHER				
	VARIATIONAL METHODS			
9.1	Hamilton's principle · · · · · · · · · · · · · · · · · · ·			
9.2	Flexural vibrations of slender beams4			
9.3	Equation of motion for honeycomb beams in flexure · · · · · · · · · · · · · · · · · · ·			
9.4	Plates with constrained viscoelastic layer · · · · · · · 15			
9.5	Timoshenko beams			
9.6	Mindlin plates			
9.7	Cylindrical shells			
9.8	Lagrange's equation · · · · · · · 34			
9.9	Garlekin's method			
9.10	An example using Garlekin's method · · · · · · 39			
Prob	lems · · · · · · · · · · · · · · · · · · ·			
Chapte	r 10 STRUCTURAL COUPLING BETWEEN			
	SIMPLE SYSTEMS 43			
10.1	Introduction			
10.2	Coupled mass-spring systems · · · · · · · · · · · · · · · · · · ·			
10.3	Coupled systems with losses · · · · · · · · · · · · · · · · · ·			
10.4	Example			
10.5	Rubber mounts, some material parameters · · · · · · · 56			
10.6	Wave propagation in rubber mounts, approximate solutions · · 62			
10.7	Equivalent stiffness of simple mounts–approximate methods $\cdot \cdot \cdot 65$			
10.8	Static deflection of cylindrical rubber mounts			
10.9	Wave propagation in circular rods, exact solutions · · · · · · · · 71			
10.10	Measurements of effective stiffness of mounts · · · · · · 80			
10.11	Structural coupling via resilient mounts · · · · · · · · · · · · · · · · · · ·			
10.19	Simple transmission model			

	10.13	Multi-point coupling · · · · · · · · · · · · · · · · · · ·
	10.14	Multi-point coupling, low and high frequency limits · · · · · · 104
	10.15	Source strength
	Proble	ems······108
Ch	apter	11 WAVES IN FLUIDS
	11.1	Wave equation · · · · · · · · · · · · · · · · · · ·
	11.2	Energy and intensity · · · · · · · · · · · · · · · · · · ·
	11.3	Losses
	11.4	Basic solutions to wave equation $\cdots 121$
	11.5	Green's function · · · · · · · · · · · · · · · · · · ·
	11.6	Dipole and other multipole sources $\cdots 129$
	11.7	Additional sources and solutions · · · · · · · · · · · · · · · · · · ·
	11.8	Moving monopole sources · · · · · · · · · · · · · · · · · · ·
	11.9	Reflection from a plane surface $\cdots 143$
	11.10	Reflection from a water surface · · · · · · · · · · · · · · · · · · 152
	11.11	Influence of temperature and velocity gradients · · · · · · · 154
	11.12	Acoustic fields in closed rooms · · · · · · · · · · · · · · · · · · ·
	11.13	Geometrical acoustics · · · · · · · · · · · · · · · · · · ·
	11.14	Near and reverberant acoustic fields in a room · · · · · · · · · · · 167
	11.15	100
		ems171
Ch	apter	12 FLUID STRUCTURE INTERACTION AND
		RADIATION OF SOUND · · · · · · 174
	12.1	Radiation and fluid loading of infinite plates · · · · · · · · 174
	12.2	Radiation—general formulation · · · · · · · · · · · · · · · · 181
	12.3	Green's function—rigid plane boundary · · · · · · · · 183
	12.4	Spatial Fourier transforms—several variables · · · · · · · 186
	12.5	Radiation from infinite point excited plates · · · · · · · · · · · 189
	12.6	Mobilities of fluid loaded infinite plates · · · · · · · · · · · · · · · · 193
	12.7	Discussion of results—infinite fluid loaded plates · · · · · · · · 196
	12.8	Radiation from finite baffled plates · · · · · · · 199
	12.9	Radiation ratios—finite baffled plates · · · · · · 205
	12.10	Radiation from point excited plates · · · · · · 210
	12.11	Sound radiation ratios—cylinders · · · · · · · · · · · · · · · · · · ·
	12.12	Losses due to radiation · · · · · · · · · · · · · · · · · · ·
	12.13	Radiation from fluid loaded finite plates · · · · · · · · · · · · · · · · 219

	Proble	ems228			
Chapter 13 SOUND TRANSMISSION LOSS OF PANELS · 22					
	13.1	Sound transmission through infinite flat panels · · · · · · · · · 230			
	13.2	Plate velocity induced by an acoustic field · · · · · · · · · · · · 238			
	13.3	Sound transmission between rooms separated by a single			
		leaf panel · · · · · · · · · · · · · · · · · · ·			
	13.4	Sound transmission between equal rooms $\cdots \cdots 249$			
	13.5	Sound transmission between irregular rooms $\cdots 251$			
	13.6	Effect of boundary conditions of plate on sound transmission			
		$loss \cdots 252$			
	13.7	Effect of a baffle on sound transmission loss $\cdots \cdots 259$			
	13.8	$Measurement\ results \cdots \cdots 264$			
	13.9	Loss factors and summary $\cdots \cdots 269$			
	13.10	Sound transmission through complex structures $\cdots 272$			
	13.11	Flanking transmission · · · · · · · · · · · · · · · · · · ·			
	13.12	Sound transmission through fluid loaded plates · · · · · · · · · · · · 277			
	Proble	ems278			
Cł	apter	14 WAVEGUIDES			
	14.1	$Introduction \cdots \cdots 281$			
	14.2	Structural waveguides · · · · · · · 282			
	14.3	Coupled structural waveguides $\cdots 285$			
	14.4	Measurements and predictions $\cdots \cdots 291$			
	14.5	Composite, sandwich and honeycomb plates · · · · · · · · · · 306			
	14.6	Flexural vibrations of honeycomb/sandwich beams · · · · · · · · 311			
	14.7	Wavenumbers, sandwich/honeycomb beams · · · · · · · · 313			
	14.8	Displacement			
	14.9	Dynamic properties of sandwich beams			
	14.10	Bending stiffness of sandwich plates · · · · · · · · 324			
	14.11	Response of sandwich beams · · · · · · · · · · · · · · · · · · ·			
	14.12	Energy flow in sandwich beams · · · · · · · · · · · · · · · · · · ·			
	14.13				
	14.14	Wave propagation on infinite cylinders · · · · · · · · · · · · · · · · 335			
	14.15	Vibration of open circular cylindrical shells · · · · · · · · · · · · · · · · · ·			
	14.16	Sound transmission loss of shallow shell segments · · · · · · · · · · · · · · · · · · ·			
	14.17	Comparison between measured and predicted TL·····348			

Problems · · · · · · · · · · · · · · · · · · ·	357
Chapter 15 RANDOM EXCITATION OF STRUCTURES	358
15.1 Introduction	
15.2 Excitation of plates · · · · · · · · · · · · · · · · · · ·	
15.3 Rain on the roof excitation of plates	
15.4 Turbulent boundary layers · · · · · · · · · · · · · · · · · · ·	
15.5 TBL models	
15.6 Plate response due to TBL excitation · · · · · · · · · · · · · · · · · · ·	
15.7 Measurements of TBL induced vibrations	
15.8 Comparison between measured and predicted velocity levels	900
induced by TBL·····	200
15.9 Parameter study	
15.10 Flow noise induced in ships	
Problems · · · · · · · · · · · · · · · · · · ·	399
Chapter 16 TRANSMISSION OF SOUND IN BUILT-UP	300
STRUCTURES·····	101
16.1 Introduction	
16.2 Statistical energy analysis, SEA · · · · · · · · · · · · · · · · · · ·	
16.3 Energy flow between continuous systems	
16.4 Coupling between acoustic fields and vibrating structures4	
16.5 Prediction of sound transmission through a panel	
using SEA · · · · · · · · · · · · · · · · · · ·	117
16.6 Sound transmission through double walls	120
16.7 Limitation of SEA derived sound transmission loss 4	123
16.8 Coupling between vibrating structures	25
16.9 Energy flow in large structures, SEA · · · · · · 4	27
16.10 SEA parameters	
16.11 Ship noise	
16.12 Waveguide model	and the second
16.13 Noise levels in accommodation spaces · · · · · 4	
16.14 Source data	
16.15 Measured and predicted results · · · · · · 4	
16.16 Conclusions—noise prediction on ships · · · · · · 4	51
Problems4	
REFERENCES4	54
Appendix A SOUND TRANSMISSION LOSS OF SINGLE	

	LEAF PANELS 471
Appendix B	VELOCITY LEVEL OF SINGLE LEAF PANELS
	EXCITED BY AN ACOUSTIC FIELD 475
Appendix C	INPUT DATA FOR NOISE PREDICTION ON
	SHIPS 478
INDEX·····	

#### Chapter 9

### HAMILTON'S PRINCIPLE AND SOME OTHER VARIATIONAL METHODS

Many problems in mathematical physics and thus in vibro-acoustics cannot be solved exactly. However, a variational technique can often be used to sufficiently well formulate the equations governing the response of a structure excited by external forces. The technique ensures that errors are minimized. Variational techniques are excellent tools for solving dynamic problems for which exact solutions cannot be formulated.

The widely used Finite Element Method, is based on Hamilton's principle, which is a very powerful variational method. The principle can be proved based on Newton's law of motion. Inversely Newton's law can be derived using Hamilton's principle. However, Hamilton's principle is much more general than Newton's law and for this reason, it has survived the revolution in mechanics brought by Einstein.

The key problem for the successful application of any variational technique is the mathematical formulation of the kinetic and potential energies of a system. This formulation also requires a physical understanding of the mechanisms governing the motion of a system. This can be illustrated by considering two different types of three layered beams. In one case, the structure consists of a beam with a constrained viscoelastic layer. For the vibrating beam, the shear forces in the viscoelastic layer along the axis of the beam are of importance. In the other case, the core of a three-layered beam consists of a honeycomb structure. In this case, the shear forces perpendicular to the axis of the beam are of major importance for the deflection of the beam. For the two cases, the energies are modelled in different ways resulting in two different equations as discussed in Sections 9.3 and 9.4. In

2 Vibro-Acoustics

each case, the results are only valid as long as the basic physical assumptions are satisfied.

Hamilton's principle is in this chapter used to derive the equations, which up to certain frequencies govern the flexural vibrations of thick beams or plates and of cylindrical shells. The Lagrange and Garlekin methods are also discussed. The longitudinal vibration of thick beams or rods is examined in Chapter 10 in connection with discussions on various models describing the axial vibration of cylindrical rubber mounts.

#### 9.1 Hamilton's principle

The most general formulation of the law governing the motion of a mechanical system is Hamilton's principle. In formulating the principle, it is assumed that the potential energy  $\mathcal{U}$  is a known function of some generalized coordinates  $q_1, q_2, \dots, q_n$  and time t. The potential energy is symbolically written as  $\mathcal{U} = \mathcal{U}(q,t)$ . The kinetic energy  $\mathcal{T}$  of the same system is also assumed to be a known function of the coordinates  $q_1, q_2, \dots, q_n$ , the velocities  $\dot{q}_1, \dot{q}_2, \dots, q_n$  and time t. Thus, the kinetic energy is written as  $\mathcal{T} = \mathcal{T}(q, \dot{q}, t)$ .

The Hamilton's principle states: between two instants of time,  $t_1$  and  $t_2$ , the motion of a mechanical system is such that for the coordinates defining the system to be described by the functions  $q_i(t)$  the integral

$$\mathcal{J} = \int_{t_1}^{t_2} (\mathcal{T} - \mathcal{V}) dt \tag{9-1}$$

is stationary. It assumed that the coordinates or displacements of the system at  $t = t_1$  and  $t = t_2$  are known.

Hamilton's principle can also be written in the form

$$\delta \int_{t_1}^{t_2} (\mathcal{T} - \mathcal{V}) \, \mathrm{d}t = 0 \tag{9-2}$$

The expression states that the variation of the integral is zero when the system is given a virtual displacement if the virtual displacement is zero at  $t = t_1$  and  $t = t_2$ . During time period  $t_1$  to  $t_2$  the system will move in such a way that the time average of the difference between the kinetic and potential energies is an extrenum or in most cases a minimum.

The difference between the kinetic and potential energies is called the Lagrangian of the system and is defined as (or sometimes as  $-\mathcal{L}$ )

$$\mathcal{L} = \mathcal{T} - \mathcal{U} \tag{9-3}$$

The influence of an external field or force can also be incorporated in the variational expression. By defining the potential energy for the conservative external forces as A and by including this in the original expression (9-2) Hamilton's principle reads

$$\delta \int_{t_1}^{t_2} (\mathcal{T} - \mathcal{U} - \mathcal{A}) dt = 0 \tag{9-4}$$

For a conservative (no losses) and external force described by the vector F acting on a particle and moving the particle along a path given by the vector s the potential energy of the external force is reduced and giving the energy as

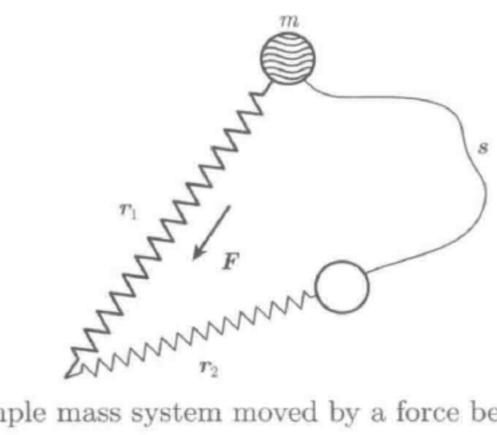
$$A = -\int F ds \qquad (9-5)$$

Hamilton's principle as a tool for deriving the governing equations describing the motion of a simple beam and some more complicated structures are discussed in the following sections.

One proof of Hamilton's principle can, as suggested by Petyt in ref. [63], be illustrated by considering a simple system shown in Fig. 9-1. Let the vector F describe a conservative force acting on the point mass m. The work  $\mathcal{W}$  done by a conservative force, defined by the vector  $\mathbf{F}$ , when moving the mass m from a position  $r_1$  to  $r_2$  is independent of the path taken. The work W done along any path s is

$$\mathcal{W} = \int m{F} \mathrm{d} m{s}$$

During the process the mass or the system has lost the potential energy  $\mathcal{U}$ , thus U = -W. The conservative vector force F can thus be expressed as a



A simple mass system moved by a force between two positions

4 Vibro-Acoustics

function of the potential energy  $\mathcal{U}$  of the symbol

$$F = -\text{grad } \mathcal{V} \tag{9-6}$$

Consider now a simple mass m subjected to a conservative force defined by the vector F. According to the principle of virtual displacement, it follows that

$$\mathbf{F} \cdot \delta \mathbf{r} - m\ddot{\mathbf{r}} \cdot \delta \mathbf{r} = 0 \tag{9-7}$$

The vector  $\mathbf{r}$  defines the position of the mass. The virtual work  $\delta \mathcal{W} = \mathbf{F} \cdot \delta \mathbf{r}$  done by the conservative force is also given by  $\delta \mathcal{V} = -\mathbf{F} \cdot \delta \mathbf{r}$  when  $\mathcal{V}$  is the potential energy of the system. Considering this, eq. (9-7) is written as

$$\delta U = -m\ddot{r} \cdot \delta r \tag{9-8}$$

The kinetic energy  $\mathcal{T}$  of the mass or in fact the system is  $\mathcal{T} = m \cdot \dot{r}^2/2$ . Thus

$$\delta T = m \cdot \dot{r} \cdot \delta \dot{r} \tag{9-9}$$

By subtracting the expression (9-8) from (9-9) the result is

$$\delta T - \delta U = m \cdot \dot{r} \cdot \delta \dot{r} + m \ddot{r} \cdot \delta r = \frac{d}{dt} (m \dot{r} \cdot \delta r)$$
 (9-10)

Integration with respect to time from  $t_1$  to  $t_2$  gives

$$\int_{t_1}^{t_2} (\delta T - \delta v) dt = [m\dot{\boldsymbol{r}} \cdot \delta \boldsymbol{r}]_{t_1}^{t_2}$$
(9-11)

Assuming that the virtual displacement  $\delta r$  is equal to zero for  $t=t_1$  and  $t=t_2$  it follows that

$$\delta \int_{t_1}^{t_2} (\mathcal{T} - \mathcal{U}) \, \mathrm{d}t = 0$$

This is Hamilton's result as given by eq. (9-2).

Hamilton's principle is discussed in for example refs. [58], [77] and [78].

#### 9.2 Flexural vibrations of slender beams

The flexural vibrations of "thin" or slender beams were discussed in Chapter 7(Volume I). Again, a "thin" beam under flexure is considered to demonstrate how Hamilton's principle can be used to derive the equations governing the motion of the beam as well as to formulate the boundary conditions of the beam.

A simple homogeneous and slender beam is shown in Fig. 9-2. The bending stiffness of the beam is D' and it's mass per unit length m'. The beam is extended along the x-axis from x = 0 to x = L and is excited by a force F'(x,t) per unit length. The resulting displacement of the beam is w(x,t) defined positive, as the force, along the positive y-axis. The forces and bending moments at the boundaries are  $F_1, F_2, M_1$  and  $M_2$  as defined in Fig. 9-2.

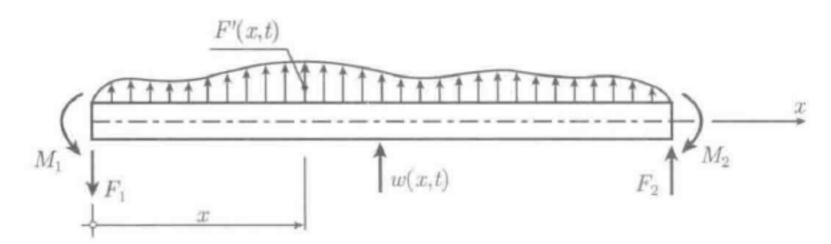


Fig. 9-2 Forces and bending moments acting on a beam

The potential energy  $U_l$  induced by bending is per unit length of the beam given by eq. (3-84) as

$$\upsilon_l = \frac{D'}{2} \cdot \left(\frac{\partial^2 w}{\partial x^2}\right)^2$$

The kinetic energy per unit length of the beam is

$$\mathcal{T}_l = \frac{m'}{2} \cdot \left(\frac{\partial w}{\partial t}\right)^2$$

The potential energy  $A_1$  per unit length induced by the external force F' is according to the definition (9-5) given by

$$\mathcal{A}_1(t) = -\int_0^L w(x, t) F'(x, t) dx$$

The potential energy  $A_2$  induced by the external forces and moments is

$$\mathcal{A}_{2}(t) = -F_{2}(t)w(L,t) + F_{1}(t)w(0,t) + M_{2}(t) \left[\frac{\partial w}{\partial x}\right]_{x=L} - M_{1}(t) \left[\frac{\partial w}{\partial x}\right]_{x=0}$$

$$= -\left[Fw - M \cdot \frac{\partial w}{\partial x}\right]_{0}^{L} \qquad (9-12)$$

The total potential energy invoked by all external forces is consequently

$$\mathcal{A}(t) = -\int_{0}^{L} w(x, t)F'(x, t)dx - \left[Fw - M \cdot \frac{\partial w}{\partial x}\right]_{0}^{L}$$
(9-13)

6 Vibro-Acoustics

According to Hamilton's principle the kinetic and potential energies should satisfy by inserting eqs. (9-4), (9-5). Thus

$$\delta \int_{t_1}^{t_2} (\mathcal{T} - \mathcal{U} - \mathcal{A}) dt = \delta \int_{t_1}^{t_2} dt \left[ -\mathcal{A} + \int_0^L dx (\mathcal{T}_l - U_l) \right] = 0$$
 (9-14)

By inserting eqs. (9-4), (9-5) and (9-12) in eq. (9-13) the result is

$$\delta \int_{t_1}^{t_2} dt \left[ \int_0^L dx \left\{ \frac{m'}{2} \cdot \left( \frac{\partial w}{\partial t} \right)^2 - \frac{D'}{2} \cdot \left( \frac{\partial^2 w}{\partial x^2} \right)^2 + wF' \right\} \right] + \left[ Fw - M \cdot \frac{\partial w}{\partial x} \right]_0^L = 0$$
(9-15)

Effecting the variation the expression (9-15) is written as

$$\int_{t_1}^{t_2} dt \left[ \int_0^L dx \left\{ m' \cdot \left( \frac{\partial w}{\partial t} \right) \left( \frac{\partial \delta w}{\partial t} \right) - D' \cdot \left( \frac{\partial^2 w}{\partial x^2} \right) \left( \frac{\partial^2 \delta w}{\partial x^2} \right) + F' \delta w \right\} \right] 
+ \int_{t_1}^{t_2} dt \left[ F \delta w - M \cdot \frac{\partial \delta w}{\partial x} \right]_0^L = 0$$
(9-16)

Next, integration by parts is carried out. For simplicity, each part of the integrand is treated separately. The first part of the double integral of (9-16) gives

$$X_{1} = \int_{0}^{L} dx \int_{t_{1}}^{t_{2}} dt \cdot m' \frac{\partial w}{\partial t} \frac{\partial \delta w}{\partial t} = \int_{0}^{L} dx \left\{ \left[ m' \frac{\partial w}{\partial t} \delta w \right]_{t_{1}}^{t_{2}} - \int_{t_{1}}^{t_{2}} dt \cdot m' \frac{\partial^{2} w}{\partial t^{2}} \delta w \right\}$$

However, as required, the displacement w is fixed at the initial and final time limits. Thus  $\delta w = 0$  at  $t = t_1$  and  $t = t_2$ . The integral  $X_1$  is consequently reduced to

$$X_1 = -\int_{t_1}^{t_2} dt \cdot m' \frac{\partial^2 w}{\partial t^2} \delta w \tag{9-17}$$

The second part  $X_2$  of the integral (9-16) is integrated by parts as

$$X_{2} = \int_{t_{1}}^{t_{2}} dt \left\{ \int_{0}^{L} dx \left[ -D' \cdot \left( \frac{\partial^{2} w}{\partial x^{2}} \right) \left( \frac{\partial^{2} \delta w}{\partial x^{2}} \right) \right] \right\}$$
$$= -D' \int_{t_{1}}^{t_{2}} dt \left[ \left( \frac{\partial^{2} w}{\partial x^{2}} \right) \left( \frac{\partial \delta w}{\partial x} \right) \right]$$

$$-\left(\frac{\partial^3 w}{\partial x^3}\right)\delta w\bigg]_0^L - D' \int_0^L \mathrm{d}x \int_{t_1}^{t_2} \mathrm{d}t \cdot \frac{\partial^4 w}{\partial x^4} \cdot \delta w \tag{9-18}$$

By introducing the expressions (9-17) and (9-18) in eq. (9-16) the result is

$$\int_{t_1}^{t_2} dt \int_{0}^{L} dx \delta w \left[ -m' \frac{\partial^2 w}{\partial t^2} - D' \frac{\partial^4 w}{\partial x^4} + F' \right] 
+ \int_{t_1}^{t_2} dt \left[ \delta w \left( D' \frac{\partial^3 w}{\partial x^3} + F \right) - \frac{\partial \delta w}{\partial x} \left( D' \frac{\partial^2 w}{\partial x^2} + M \right) \right]_{0}^{L} = 0 \quad (9-19)$$

For the result to be zero for any  $\delta w$  or  $\partial \delta w/\partial x$ , it follows that the expressions inside the brackets must be zero. Setting the expression inside the first bracket equal to zero gives

$$D'\frac{\partial^4 w}{\partial x^4} + m'\frac{\partial^2 w}{\partial t^2} = F' \qquad (9-20)$$

This is the equation of motion of a slender and homogeneous beam in flexure as already discussed in Section 3.7.

By setting the second square bracket equal zero, the boundary conditions for x=0 and x=L are obtained as

$$\delta w \left( D' \frac{\partial^3 w}{\partial x^3} + F \right) = 0 \tag{9-21}$$

$$\frac{\partial \delta w}{\partial x} \left( D' \frac{\partial^2 w}{\partial x^2} + M \right) = 0 \tag{9-22}$$

For the first condition (9-21) to be satisfied at a boundary, it follows that

$$F = -D' \frac{\partial^3 w}{\partial x^3}$$
 or  $\delta w = 0$  (9-23)

The second condition requires

$$M = -D' \frac{\partial^2 w}{\partial x^2}$$
 or  $\frac{\partial \delta w}{\partial x} = 0$  (9-24)

The results give the expressions for force and bending moment as given by the displacement w of the beam as already derived in Chapter 3, eqs.(3-73) and (3-75). The conditions  $\delta w = 0$  and  $\delta(\partial w/\partial x) = 0$  are equivalent to w and  $\partial w/\partial x$  being constant at the boundaries. For most practical purposes a coordinate system can be oriented in such a way that the boundary conditions can be written w = 0 and  $\partial w/\partial x = 0$ .