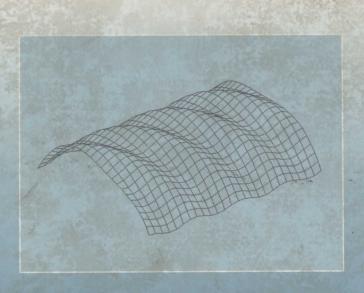
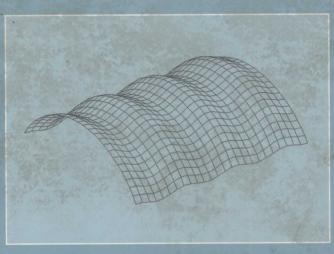
László P. Kollár George S. Springer

MECHANICS OF Composite Structures





CAMBRIDGE

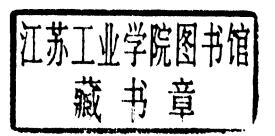
MECHANICS OF COMPOSITE STRUCTURES

LÁSZLÓ P. KOLLÁR

Budapest University of Technology and Economics

GEORGE S. SPRINGER

Stanford University





PUBLISHED BY THE PRESS SYNDICATE OF THE UNIVERSITY OF CAMBRIDGE The Pitt Building, Trumpington Street, Cambridge, United Kingdom

CAMBRIDGE UNIVERSITY PRESS
The Edinburgh Building, Cambridge CB2 2RU, UK
40 West 20th Street, New York, NY 10011-4211, USA
477 Williamstown Road, Port Melbourne, VIC 3207, Australia
Ruiz de Alarcón 13, 28014 Madrid, Spain
Dock House, The Waterfront, Cape Town 8001, South Africa
http://www.cambridge.org

© Cambridge University Press 2003

This book is in copyright. Subject to statutory exception and to the provisions of relevant collective licensing agreements, no reproduction of any part may take place without the written permission of Cambridge University Press.

First published 2003

Printed in the United States of America

Typefaces Times Ten 10/13.5 pt. and Helvetica Neue Condensed System LATEX 2_E [TB]

A catalog record for this book is available from the British Library.

Library of Congress Cataloging in Publication Data

Kollar, L. Peter (Laszlo Peter), 1926-

Mechanics of composite structures / László P. Kollár, George S. Springer.

p. cm.

Includes bibliographical references and index.

ISBN 0-521-80165-6

1. Composite materials – Mechanical properties. I. Springer, George S. II. Title. TA418.9.C6 K5875 2002

624.1'8 - dc21

2002034796

ISBN 0 521 80165 6 hardback

Preface

The increased use of composites in aerospace, land, and marine applications has resulted in a growing demand for engineers versed in the design of structures made of fiber-reinforced composite materials. To satisfy this demand, and to introduce engineers to the subject of composites, numerous excellent texts have been published dealing with the mechanics of composites. These texts deal with those fundamental aspects needed by engineers new to the subject. Our book addresses topics not generally covered by existing texts but that are necessary for designing practical structures. Among the topics in this book of special interest to the designer, but that usually are not included in standard texts, are stress-strain relationships for a wide range of anisotropic materials; bending, buckling, and vibration of plates; bending, torsion, buckling, and vibration of solid as well as thin-walled beams; shells; hygrothermal stresses and strains; and finite element formulation. The material is presented in sufficient detail to enable the reader to follow the developments leading to the final results. The expressions resulting from the analyses are either readily usable or can be translated into a computer algorithm. Thus, the book should be useful to students and researchers wishing to acquire knowledge of some of the advanced concepts of the mechanics of composites as well as to engineers engaged in the design of structures made of composite materials.

The emphasis is on analyses built on fundamental concepts that are applicable to a variety of structural design problems. In presenting the material we have strived to follow the outline commonly used in texts dealing with the analysis of structures made of isotropic materials. We have consciously omitted empirical approaches. Test results are certainly of value to the engineer. However, for composites, these mostly apply only under specific circumstances and cannot readily be generalized to different materials and different applications. We have included material properties data to help the designer perform calculations without the need to search the literature.

The book is self-contained. Nevertheless, the reader will find it helpful to have a background in mechanics and in composites and some knowledge of differential xii PREFACE

equations and matrix algebra. We have made an effort to keep the notation as uniform as practicable and reasonably consistent with accepted usage. The principal symbols are summarized in a list of symbols.

We are grateful to Professor István Hegedűs for his constructive comments. We thank Dr. Rita Kiss, Gabriella Tarján, and Anikó Pluzsik for proofreading portions of the manuscript, Gabriella Tarján for preparing the illustrations, and Eric Allison and Sarah Brennan for their help in compiling the index.

László P. Kollár Budapest

George S. Springer Stanford

List of Symbols

We have used, wherever possible, notation standard in elasticity, structural analysis, and composite materials. We tried to avoid duplication, although there is some repetition of those symbols that are used only locally. In the following list we have not included those symbols that pertain only to the local discussion. Below, we give a verbal description of each symbol and, when appropriate, the number of the equation in which the symbol is first used.

Latin letters

```
\boldsymbol{A}
                 area
       Aiso
                 tensile stiffness of an isotropic laminate (Eq. 3.42)
 [A], A_{ii}
                 tensile stiffness of a laminate (Eqs. 3.18, 3.19)
   [a], a_{ij}
                inverse of the [A] matrix for symmetric laminates (Eq. 3.29)
  [B], B_{ij}
                stiffness of a laminate (Eqs. 3.18, 3.19)
  [C], C_{ii}
                3D stiffness matrix in the x_1, x_2, x_3 coordinate system (Eq. 2.22)
  [\overline{C}], \overline{C}_{ij}
                3D stiffness matrix in the x, y, z coordinate system (Eq. 2.19)
                moisture concentration (Eq. 2.154); core thickness (Fig. 5.2)
  [D], D_{ii}
                bending stiffness of a laminate (Eqs. 3.18, 3.19)
 [D]^*, D_{ii}^*
                reduced bending stiffness of a laminate (Eq. 4.1)
       Diso
                bending stiffness of an isotropic laminate (Eq. 3.42)
 D, \overline{D}, \widehat{D}
                parameters (Table 6.2, page 222, Eq. 6.157)
   [d], d_{ij}
                inverse of the [D] matrix for symmetrical laminates (Eq. 3.30)
  d, d^{t}, d^{b}
                distances for sandwich plates (Fig. 5.2)
E_1, E_2, E_3
                Young's moduli in the x_1, x_2, x_3 coordinate system (Table 2.5)
       [E]
                stiffness matrix in the FE calculation (Eq. 9.4)
       \widehat{EA}
                tensile stiffness of a beam (Eq. 6.8)
        \widehat{EI}
                bending stiffness of a beam (Eq. 6.8)
       \widehat{EL}_{\omega}
                warping stiffness of a beam (Eq. 6.244)
    F_i, F_{ij}
                strength parameters in the quadratic failure criterion (Eq. 10.2)
```

xiv LIST OF SYMBOLS

f_{ij}	constants in the quadratic failure criterion (Eq. 10.25)
f, f_{ij}	frequency (Eq. 4.190)
f_x , f_y , f_z ,	body forces per unit volume (Eq. 2.13)
G_{23},G_{13},G_{12}	shear moduli in the x_1 , x_2 , x_3 coordinate system (Table 2.5)
$\widehat{GI}_{\mathfrak{t}}$	torsional stiffness of a beam (Eq. 6.8)
h	plate thickness
$h_{\mathrm{b}},h_{\mathrm{t}}$	distances of the bottom and top surfaces of a plate from the
	reference plane (Eq. 3.9)
i_{ω}	polar radius of gyration (Eq. 6.340)
[J]	inverse of the material stiffness matrix $[E]$ (Eq. 9.16)
K	number of layers in a laminate; number of wall segments;
	stiffness parameter of a plate (Eq. 4.153)
\widetilde{k}	rotational spring constant (Eq. 4.149)
\boldsymbol{k}	equivalent length factor (Eq. 6.340)
L_x , L_y	dimensions of a plate
$\stackrel{\cdot}{L}$	length; number of cells in a multicell beam (Eq. 6.222)
L_i , $L_i^{ m f}$	load and failure load (Eq. 10.42)
l_x, l_x^{o}	half buckling length (Eq. 4.142), half buckling length
.	corresponding to the lowest buckling load of a long plate
	(Eq. 4.173)
M_x , M_y , M_{xy}	bending and twist moments per unit length acting on a
,	laminate (Eq. 3.9)
$M_{\rm r}^{\rm ht}$, $M_{\rm v}^{\rm ht}$, $M_{\rm rv}^{\rm ht}$	hygrothermal moments per unit length (Eq. 4.247)
$M_x^{ m ht}, M_y^{ m ht}, M_{xy}^{ m ht}$ $\widehat{M}_y, \widehat{M}_z$	bending moments acting on a beam (Fig. 6.2)
\widehat{M}_{ω}	bimoment acting on a beam (Eq. 6.232)
N_x , N_y , N_{xy}	in-plane forces per unit length acting on a laminate (Eq. 3.9)
N_{x0} , N_{y0} , N_{xy0}	in-plane compressive forces per unit length (Eq. 4.109)
$N_x^{\rm ht}$, $N_y^{\rm ht}$, $N_{xy}^{\rm ht}$	hygrothermal forces per unit length (Eq. 4.246)
$N_{x, cr}$	buckling load of a uniaxially loaded plate (Eq. 4.141)
$N_{ extsf{x, cr}} \ \widehat{N} \ \widehat{N}_{ extsf{cr}}, \ \widehat{N}_{ extsf{cr}}^{ extsf{B}}$	axial force acting on a beam (Fig. 6.2)
$\widehat{N}_{ m cr}, \widehat{N}_{ m cr}^{ m B}$	buckling load and buckling load due to bending deformation
	(Eq. 6.337)
$\widehat{N}_{ ext{cry}},\widehat{N}_{ ext{crz}}$	buckling load in the $x-z$ and $x-y$ planes, respectively
	(Eqs. 6.337, 7.110)
$\widehat{N}_{{ m cr}\psi}$	buckling load under torsional buckling (Eqs. 6.337, 7.110)
$[P], [\overline{\overline{P}}]$	stiffness matrix of a beam (Eqs. 6.2, 6.250). Without bar refers
	to the centroid; with bar to an arbitrarily chosen coordinate
	system
p	transverse load per unit area; distance between the origin and
•	the tangent of the wall of a beam (Eq. 6.190)
p_x, p_y, p_z	axial and transverse loads (per unit length) acting on a beam
1 W/ 1 Y 1 1 6	(Fig. 6.1); surface forces per unit area (Eq. 2.166)
$[\mathit{Q}], \mathit{Q}_{ij}$	2D plane-stress stiffness matrix in the x_1 , x_2 coordinate system
[2], 2.1	(Eq. 2.134)
	V T - 7

X۷ LIST OF SYMBOLS

```
2D plane-stress stiffness matrix in the x, y coordinate system
   [\overline{Q}], \overline{Q}_{ij}
                   (Eq. 2.126)
                   buckling load resulting in lateral buckling (Eq. 6.359)
                   shear flow (Eq. 6.189).
                   stiffness parameter (Eq. 3.46)
                   stress ratio (Eq. 10.42)
                   radii of curvatures of a shell (Eq. 8.1)
R_x, R_y, R_{xy}
                   compliance matrix under plane-strain condition in the x_1, x_2
    [R], R_{ij}
                   coordinate system (Eq. 2.79)
    [\overline{R}], \overline{R}_{ii}
                   compliance matrix under plane-strain condition in the x, y
                   coordinate system (Eq. 2.65)
                   3D compliance matrix in the x_1, x_2, x_3 coordinate system
     [S], S_{ij}
                   (Eq. 2.23)
     [\overline{S}], \overline{S}_{ij}
                   3D compliance matrix in the x, y, z coordinate system
                   (Eq. 2.21)
 \begin{array}{c} \widehat{S}_{ij} \\ \widetilde{S}_{ij} \\ \widehat{s}_{ij} \\ \widehat{s}_{ij} \\ s_1^+, s_2^+, s_3^+ \end{array}
                   shear stiffness of a beam, i, j = z, y, \omega (Eqs. 7.13, 7.36)
                   shear stiffness of a plate, i, j = 1, 2 (Eq. 5.15)
                   shear compliance of a beam, i, j = z, y, \omega (Eq. 7.38)
                   tensile strengths (Eq. 10.13)
 s_1^-, s_2^-, s_3^-
                   compression strengths (Eq. 10.13)
s_{23}, s_{13}, s_{12}
\widehat{T}
                   shear strengths (Eq. 10.15)
                   torque acting on a beam (Fig. 6.2)
                   restrained warping-induced torque (Eq. 6.235)
          \widehat{T}_{\mathrm{sv}}
                    Saint-Venant torque (Eq. 6.239)
         [T_{\sigma}]
                    2D stress transformation matrix (Eq. 2.182)
                    3D stress transformation matrix (Eq. 2.179)
         [\hat{T}_{\sigma}]
                    2D strain transformation matrix (Eq. 2.188)
         [T_{\epsilon}]
                    3D strain transformation matrix (Eq. 2.185)
         [\hat{T}_{\epsilon}]
                    torque load acting on a beam (Fig. 6.1)
                    thicknesses of the top and bottom facesheets (Eq. 5.26)
        t^{t}, t^{b}
                    strain energy (Eq. 2.200)
            \boldsymbol{U}
                    displacement in the x direction; varies with the x and y
            \boldsymbol{U}
                    coordinates only (Eq. 2.50)
                    displacement in the x direction
            и
                    displacement of the reference surface in the x direction
           u^{o}
                    displacements in the x_1, x_2, and x_3 direction
   u_1, u_2, u_3
                    displacement in the y direction; varies with the x and y
            V
                    coordinates only (Eq. 2.51)
   V_{\rm f}, V_{\rm m}, V_{\rm v}
                    volume of fibers, matrix, and void
       V_x, V_y \ \widehat{V}_y, \widehat{V}_z
                    out-of-plane shear forces per unit length (Eq. 3.10)
                    transverse shear forces acting on a beam (Fig. 6.2)
                    displacement in the y direction
            v^{o}
                    displacement of the reference surface in the y direction
                    volume fraction of fibers, matrix, and void
```

 $v_{\rm f}, v_{\rm m}, v_{\rm v}$

χVÍ **LIST OF SYMBOLS**

> Wdisplacement in the z direction; varies with the x and ycoordinates only (Eq. 2.52) $[W], [\overline{W}]$ compliance matrix of a beam (Eq. 6.17). No bar refers to the centroid; bar to an arbitrarily chosen coordinate system \boldsymbol{w} deflection in the z direction maximum deflection in the z direction (Eq. 4.29) \widetilde{w} w^{o} deflection of the reference surface in the z direction $w^{\rm B}, w^{\rm S}$ deflections due to bending and shear deformations (Eq. 7.85) coordinates of the centroid of a beam (Eqs. 6.54, 6.73) y_c, z_c coordinates of the shear center of a beam (Eq. 6.311) $y_{\rm sc}, z_{\rm sc}$ coordinates of the top and bottom surfaces of the kth ply in a z_k, z_{k-1} laminate (Eq. 3.20)

Greek letters

α	parameter describing shear deformation (Eq. 7.253)
$lpha_i$	parameter describing shear deformation, $i = w, \psi, N, \omega$
	(Eq. 7.244)
$\left[lpha ight],lpha_{ij}$	compliance matrix of a laminate (Eq. 3.23)
α, β	
\widehat{lpha}_{ij}	parameters describing buckled shape of a shell (Eq. 8.78)
$\widetilde{lpha}_i, \widetilde{lpha}_{ij}$	compliances for closed-section beams (Eq. 6.156)
•	thermal expansion coefficients (Eqs. 2.153, 2.158)
β, λ	parameters in the displacements of a cylinder (Eq. 8.30)
$[eta]$, \underline{eta}_{ij}	compliance matrix of a laminate (Eq. 3.23)
$egin{array}{c} \overline{eta}_{ij} \ \widehat{eta}_{ij} \ \widetilde{eta}_{i}, \ \widetilde{eta}_{ij} \end{array}$	compliance of symmetrical cross-section beams (Table 6.2)
$\sim \stackrel{eta_{ij}}{\sim}$	compliance of closed-section beams (Eq. 6.156)
eta_i, eta_{ij}	moisture expansion coefficients in the x , y , z directions
	(Eqs. 2.154, 2.159)
$oldsymbol{eta}_1$	property of the cross section (Eq. 6.360)
γ_y, γ_z	shear strain in a beam in the $x-y$ and $x-z$ planes (Eq. 7.2)
$\gamma_{yz}, \gamma_{xz}, \gamma_{xy}$	engineering shear strain in the x , y , z coordinate system
	(Eq. 2.9)
$\gamma_{23}, \gamma_{13}, \gamma_{12}$	engineering shear strain in the x_1, x_2, x_3 coordinate system
Δh	change in thickness (Eq. 4.282)
ΔT	temperature change (Eq. 2.153)
$[\delta], \delta_{ij}$	compliance matrix of a laminate (Eq. 3.23)
$\widehat{\delta}_{ij}$	compliance of closed-section beams (Eq. 6.157)
$\overline{\epsilon}_{x},\dots$	average strains in a sublaminate (Eq. 9.14)
$\epsilon_x, \epsilon_y, \epsilon_z$	engineering normal strains in the x , y , z coordinate system
$\epsilon_1, \epsilon_2, \epsilon_3$	engineering normal strains in the x_1, x_2, x_3 coordinate system
$\epsilon_x^{\rm o}, \epsilon_y^{\rm o}, \gamma_{xy}^{\rm o}$	strains of the reference surface
$\epsilon_x^{\mathrm{o,ht}}, \epsilon_y^{\mathrm{o,ht}}, \gamma_{xy}^{\mathrm{o,ht}}$	hygrothermal strains in a laminate (Eq. 4.250)
ζ	parameter of restraint (Eq. 4.152)
Θ	polar moment of mass (Eq. 6.411)
3	Posses Monte Of Intain (Lq. 0.711)

LIST OF SYMBOLS xvii

Θ_k	ply orientation
ϑ	rate of twist (Eq. 6.1)
$\vartheta^{\mathbf{B}}, \vartheta^{\mathbf{S}}$	rate of twist due to bending and shear deformation (Eq. 7.5)
$\kappa_x, \kappa_y, \kappa_{xy}$	curvatures of the reference surface (Eq. 3.8)
$\kappa_x^{\mathrm{ht}}, \kappa_y^{\mathrm{ht}}, \kappa_{xy}^{\mathrm{ht}}$	hygrothermal curvatures of a laminate (Eq. 4.250)
$\lambda, \lambda_{cr}, \lambda_{ij}$	load parameter (Eq. 4.109); buckling load parameter
	(Eq. 4.121); eigenvalue (Eq. 4.225)
$\mu_{\mathit{Bi}}, \mu_{\mathit{Gi}}, \mu_{\mathit{Si}}$	parameters in the calculation of natural frequencies
	(Eqs. 6.398, 6.400, 7.203)
$ u_{ij}$	Poisson's ratio
ξ, η, ζ	coordinates attached to the wall of a beam (Fig. 6.13)
ξ, ξ'	parameters in the expressions of the buckling loads of plates
	with rotationally restrained edges (Eq. 4.151)
$\pi_{ m p}$	potential energy (Eq. 2.204)
ρ_x, ρ_y, ρ_z	radius of curvature in the $y-z$, $x-z$, and $x-y$ planes (Eq. 2.45)
ρ_1, ρ_2, ρ_3	radius of curvature in the x_2-x_3 , x_1-x_3 , and x_1-x_2 planes
	(Eq. 2.53)
$ ho_{ m comp}, ho_{ m f}, ho_{ m m}$	densities of composite, fiber, and matrix
ho	mass per unit area or per unit length
$\sigma_1, \sigma_2, \sigma_3$	normal stresses in the x_1, x_2, x_3 coordinate system
$\sigma_x, \sigma_y, \sigma_z$	normal stresses in the x , y , z coordinate system
$\overline{\sigma}$	average stress
$\tau_{23},\tau_{13},\tau_{12}$	shear stresses in the x_1, x_2, x_3 coordinate system
$\tau_{yz}, \tau_{xz}, \tau_{yx}$	shear stresses in the x , y , z coordinate system
χ_{xz}, χ_{yz}	rotation of the normal of a plate in the $x-z$ and $x-y$ planes
	(Eqs. 3.2 and 5.1)
χ_y, χ_z	rotation of the cross section of a beam in the $x-y$ and $x-z$
	planes (Eq. 7.2)
ψ	angle of rotation of the cross section about the beam axis
	(twist) (Fig. 6.3)
Ψ	bending stiffness of an unsymmetrical long plate (Eq. 4.52)
Ω	potential energy of the external loads (Eq. 2.203)
ω	circular frequency (Eq. 4.190)
$\omega^{ m B},\omega^{ m S}$	circular frequency of a beam due to bending and shear
	deformation (Eq. 7.198)
ω_y, ω_z	circular frequency of a freely vibrating beam in the $x-z$ and
	x-y planes, respectively (Eq. 6.398)
ω_{ψ}	circular frequency of a freely vibrating beam under torsional
~ -	vibration (Eq. 6.400)
$arrho,\widetilde{arrho},ar{arrho},\widehat{arrho}$	distances between the new and the old reference surfaces
	(Eqs. 3.47, 6.105, 6.107, A.3)

Contents

Preface po		page xi
	t of Symbols	xiii
1	Introduction	1
=	Displacements, Strains, and Stresses	3
2	2.1 Strain-Displacement Relations	4
	2.1 Strain-Displacement Relations 2.2 Equilibrium Equations	6
	2.3 Stress-Strain Relationships	8
	2.3.1 Generally Anisotropic Material	8
	2.3.2 Monoclinic Material	11
	2.3.3 Orthotropic Material	14
	2.3.4 Transversely Isotropic Material	19
	2.3.5 Isotropic Material	20
	2.4 Plane–Strain Condition	22
	2.4.1 Free End – Generally Anisotropic Material	28
	2.4.2 Free End – Monoclinic Material	30
	2.4.3 Free End – Orthotropic, Transversely Isotropic,	
	or Isotropic Material	34
	2.4.4 Built-In Ends – Generally Anisotropic Material	35
	2.4.5 Built-In Ends – Monoclinic Material	36
	2.4.6 Built-In Ends – Orthotropic, Transversely Isotropic,	
	or Isotropic Material	38
	2.5 Plane-Stress Condition	38
	2.6 Hygrothermal Strains and Stresses	44
	2.6.1 Plane-Strain Condition	47
	2.6.2 Plane-Stress Condition	47
	2.7 Boundary Conditions	47
	2.8 Continuity Conditions	48
	2.9 Stress and Strain Transformations	49
	2.9.1 Stress Transformation	50
	2.9.2 Strain Transformation	52
	2.9.3 Transformation of the Stiffness and Compliance Matrices	53

VI

	2.10 Strain Energy	55
	2.10 Strain Energy 2.10.1 The Ritz Method	55
	2.11 Summary	56
	2.11 Summary 2.11 Note on the Compliance and Stiffness Matrices	56
_		63
3	Laminated Composites 3.1 Laminate Code	63
	3.2 Stiffness Matrices of Thin Laminates	65
	3.2.1 The Significance of the [A], [B], and [D] Stiffness Matrices	72
	3.2.2 Stiffness Matrices for Selected Laminates	74
		89
4	Thin Plates	90
	4.1 Governing Equations	92
	4.1.1 Boundary Conditions	92
	4.1.2 Strain Energy	93
	4.2 Deflection of Rectangular Plates	93
	4.2.1 Pure Bending and In-Plane Loads	93 94
	4.2.2 Long Plates	100
	4.2.3 Simply Supported Plates – Symmetrical Layup	100
	4.2.4 Plates with Built-In Edges - Orthotropic and	107
	Symmetrical Layup	112
	4.3 Buckling of Rectangular Plates	112
	4.3.1 Simply Supported Plates – Symmetrical Layup	112
	4.3.2 Plates with Built-In and Simply Supported Edges – Orthotropic	118
	and Symmetrical Layup	110
	4.3.3 Plates with One Free Edge – Orthotropic and	124
	Symmetrical Layup	124
	4.3.4 Plates with Rotationally Restrained Edges - Orthotropic	127
	and Symmetrical Layup	132
	4.3.5 Long Plates	141
	4.4 Free Vibration of Rectangular Plates	141
	4.4.1 Long Plates	144
	4.4.2 Simply Supported Plates – Symmetrical Layup	177
	4.4.3 Plates with Built-In and Simply Supported Edges - Orthotropic	149
	and Symmetrical Layup	151
	4.5 Hygrothermal Effects	161
	4.5.1 Change in Thickness Due to Hygrothermal Effects	163
	4.6 Plates with a Circular or an Elliptical Hole	166
	4.7 Interlaminar Stresses	
ļ	5 Sandwich Plates	169
	5.1 Governing Equations	170
	5.1.1 Boundary Conditions	172
	5.1.2 Strain Energy	173
	5.1.3 Stiffness Matrices of Sandwich Plates	174
	5.2 Deflection of Rectangular Sandwich Plates	178
	5.2.1 Long Plates	178
	5.2.2 Simply Supported Sandwich Plates - Orthotropic and	
	Symmetrical Layup	182

CONTENTS

		105
	5.3 Buckling of Rectangular Sandwich Plates	185
	5.3.1 Long Plates	185
	5.3.2 Simply Supported Plates - Orthotropic and	107
	Symmetrical Layup	187
	5.3.3 Face Wrinkling	190
	5.4 Free Vibration of Rectangular Sandwich Plates	196
	5.4.1 Long Plates	196
	5.4.2 Simply Supported Plates – Orthotropic and	
	Symmetrical Layup	199
_	•	203
6	Beams	203
	6.1 Governing Equations	205
	6.1.1 Boundary Conditions	205
	6.1.2 Stiffness Matrix	209
	6.1.3 Compliance Matrix	210
	6.1.4 Replacement Stiffnesses	
	6.2 Rectangular, Solid Beams Subjected to Axial Load	210
	and Bending	211
	6.2.1 Displacements – Symmetrical Layup	213
	6.2.2 Displacements – Unsymmetrical Layup	214
	6.2.3 Stresses and Strains6.3 Thin-Walled, Open-Section Orthotropic or Symmetrical	
	Cross-Section Beams Subjected to Axial Load and Bending	217
	Cross-Section Beams Subjected to Anial Board and Bondary	217
	6.3.1 Displacements of T-Beams	221
	6.3.2 Displacements of L-Beams	226
	6.3.3 Displacements of Arbitrary Cross-Section Beams	233
	6.3.4 Stresses and Strains6.4 Thin-Walled, Closed-Section Orthotropic Beams Subjected to	
	6.4 Thin-walled, Closed-section Orthotropic Beams subjected to	243
	Axial Load and Bending 6.5 Torsion of Thin-Walled Beams	248
	6.5 Torsion of Thin-walled Dealis	248
	6.5.1 Thin Rectangular Cross Section	250
	6.5.2 Open-Section Orthotropic Beams	252
	6.5.3 Closed-Section Orthotropic Beams – Single Cell	260
	6.5.4 Closed-Section Orthotropic Beams – Multicell	261
	6.5.5 Restrained Warping – Open-Section Orthotropic Beams	264
	6.5.6 Restrained Warping – Closed-Section Orthotropic Beams	20.
	6.6 Thin-Walled Beams with Arbitrary Layup Subjected to	265
	Axial Load, Bending, and Torsion	267
	6.6.1 Displacements of Open- and Closed-Section Beams	268
	6.6.2 Stresses and Strains in Open- and Closed-Section Beams	271
	6.6.3 Centroid	271
	6.6.4 Restrained Warping	274
	6.7 Transversely Loaded Thin-Walled Beams	
	6.7.1 Beams with Orthotropic Layup or with Symmetrical	276
	Cross Section	280
	6.7.2 Beams with Arbitrary Layup	283
	6.7.3 Shear Center	288
	6.8 Stiffened Thin-Walled Beams	200

viii CONTENTS

	6.9 Buckling of Beams	290
	6.9.1 Beams Subjected to Axial Load (Flexural-Torsional	
	Buckling)	291
	6.9.2 Lateral-Torsional Buckling of Orthotropic Beams with	
	Symmetrical Cross Section	296
	6.9.3 Local Buckling	300
	6.10 Free Vibration of Beams (Flexural-Torsional Vibration)	306
	6.10.1 Doubly Symmetrical Cross Sections	306
	6.10.2 Beams with Symmetrical Cross Sections	309
	6.10.3 Beams with Unsymmetrical Cross Sections	309
	6.11 Summary	312
7	Beams with Shear Deformation	313
_	7.1 Governing Equations	314
	7.1.1 Strain-Displacement Relationships	315
	7.1.2 Force–Strain Relationships	315
	7.1.3 Equilibrium Equations	320
	7.1.4 Summary of Equations	320
	7.1.5 Boundary Conditions	321
	7.2 Stiffnesses and Compliances of Beams	321
	7.2.1 Shear Stiffnesses and Compliances of Thin-Walled	
	Open-Section Beams	322
	7.2.2 Shear Stiffnesses and Compliances of Thin-Walled	
	Closed-Section Beams	325
	7.2.3 Stiffnesses of Sandwich Beams	326
	7.3 Transversely Loaded Beams	329
	7.4 Buckling of Beams	334
	7.4.1 Axially Loaded Beams with Doubly Symmetrical Cross	
	Sections (Flexural and Torsional Buckling)	335
	7.4.2 Axially Loaded Beams with Symmetrical or Unsymmetrical	
	Cross Sections (Flexural-Torsional Buckling)	341
	7.4.3 Lateral-Torsional Buckling of Beams with Symmetrical	
	Cross Section	345
	7.4.4 Summary	346
	7.5 Free Vibration of Beams	347
	7.5.1 Beams with Doubly Symmetrical Cross Sections	347
	7.5.2 Beams with Symmetrical or Unsymmetrical Cross Sections	356
	7.5.3 Summary	359
	7.6 Effect of Shear Deformation	359
8	Shells	365
	8.1 Shells of Revolution with Axisymmetrical Loading	367
	8.2 Cylindrical Shells	368
	8.2.1 Membrane Theory	368
	8.2.2 Built-In Ends	370
	8.2.3 Temperature – Built-In Ends	379
	8.3 Springback	380
	8.3.1 Springback of Cylindrical Shells	380
	8.3.2 Doubly Curved Shells	384

CONTENTS

	8.4 Buckling of Shells	384 387
	8.4.1 Buckling of Cylinders	
9	inite Element Analysis	395
	9.1 Three-Dimensional Element	396
	9.2 Plate Element	397
	9.3 Beam Element	397
	9.4 Sublaminate	398
	9.4.1 Step 1. Elements of $[J]$ due to In-Plane Stresses	400
	9.4.2 Step 2. Elements of $[J]$ due to Out-of-Plane	
	Normal Stresses	403
	9.4.3 Step 3. Elements of [J] due to Out-of-Plane Shear St	resses 405
	9.4.4 Step 4. The Stiffness Matrix	407
10	Failure Criteria	411
	10.1 Quadratic Failure Criterion	413
	10.1.1 Orthotropic Material	414
	10.1.2 Transversely Isotropic Material	420
	10.1.3 Isotropic Material	421
	10.1.4 Plane-Strain and Plane-Stress Conditions	422
	10.1.5 Proportional Loading – Stress Ratio	423
	10.2 "Maximum Stress" Failure Criterion	425
	10.3 "Maximum Strain" Failure Criterion	426
	10.4 Plate with a Hole or a Notch	430
	10.4.1 Plate with a Circular Hole	431
	10.4.2 Plate with a Notch	434
	10.4.3 Characteristic Length	434
11	Micromechanics	436
	11.1 Rule of Mixtures	436
	11.1.1 Longitudinal Young Modulus E_1	438
	11.1.2 Transverse Young Modulus E_2	439
	11.1.3 Longitudinal Shear Modulus G_{12}	439
	11.1.4 Transverse Shear Modulus G_{23}	440
	11.1.5 Longitudinal Poisson Ratio v_{12}	441
	11.1.6 Transverse Poisson Ratio v_{23}	442
	11.1.7 Thermal Expansion Coefficients	443
	11.1.8 Moisture Expansion Coefficients	445
	11.1.9 Thermal Conductivity	446
	11.1.10 Moisture Diffusivity	447
	11.1.11 Specific Heat	448
	11.2 Modified Rule of Mixtures	448
	11.3 Note on the Micromechanics Models	449
Appendix A. Cross-Sectional Properties of Thin-Walled Composite Beams		s 453
Appendix B. Buckling Loads and Natural Frequencies of Orthotropic Beams		ms 461
	with Shear Deformation	
A	pendix C. Typical Material Properties	464
Index		469

CHAPTER ONE

Introduction

In this book we focus on fiber-reinforced composites composed of fibers embedded in a matrix. The fibers may be short or long, continuous or discontinuous, and may be in one or in multiple directions (Fig. 1.1). Such materials offer advantages over conventional isotropic structural materials such as steel, aluminum, and other types of metal. These advantages include high strength, low weight, and good fatigue and corrosion resistance. In addition, by changing the arrangements of the fibers, the properties of the material can be tailored to meet the requirements of a specific design.

The excellent properties of composites are achieved by the favorable characteristics of the two major constituents, namely the fiber and the matrix. In low-performance composites, the reinforcements, usually in the form of short or chopped fibers (or particles), provide some stiffening but very little strengthening; the load is mainly carried by the matrix. In high-performance composites, continuous fibers provide the desirable stiffness and strength, whereas the matrix provides protection and support for the fibers, and, importantly, helps redistribute the load from broken to adjacent intact fibers.

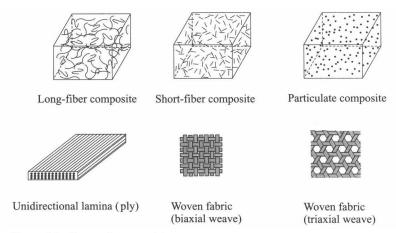


Figure 1.1: Composite material systems.

2 INTRODUCTION

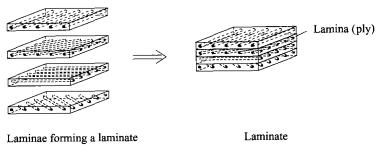


Figure 1.2: Laminated composite.

The arrangement of the fibers in a structure is governed by the structural requirements and by the process used to fabricate the part. Frequently, though not always, composite structures are made of thin layers called laminae or plies. Within each lamina, the fibers may be aligned in the same direction (unidirectional ply, Fig. 1.1) or in different directions. The latter configuration is produced, for example, by weaving the fibers in two or more directions (woven fabric). The lamina may also contain short fibers either oriented in the same direction or distributed randomly. Several laminae are then combined into a laminate to form the desired structure (Fig. 1.2).

The mechanical and thermal behaviors of a structure depend on the properties of the fibers and the matrix and on the amount and orientations of the fibers. In this book, we consider the design steps from micromechanics (which takes into account the fiber and matrix properties) through macromechanics (which treats the properties of the composite) to structural analysis. These steps are illustrated in Figure 1.3 for a structure made of laminated composite.

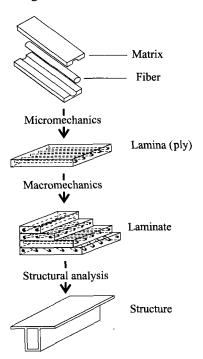


Figure 1.3: The levels of analysis for a structure made of laminated composite.