



普通高等教育土建学科专业“十一五”规划教材
高校土木工程专业规划教材

Underground Structures (地下建筑结构)

Edited By
Zixin Zhang & Xinyu Hu

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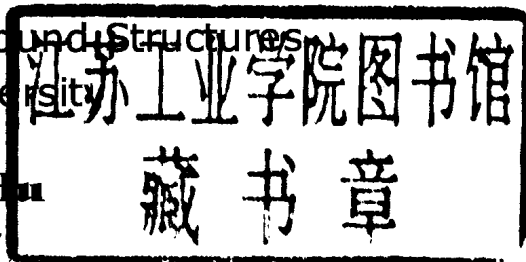
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Preface

Underground structures originated thousand years ago under the pressure of necessity. As the practical problems involving underground structures broadened in scope, the inadequacy of the scientific tools available for coping with them became increasingly apparent. Underground structures may be a tunnel, a shaft, or a cavern, either alone or a combination thereof. These texts were well received by instructors, students, and practitioners alike. The philosophy of the book was to include only items of utmost importance which will improve the quality of the final designed product, and will render a very cost effective design.

About seven years ago, there were several requests to prepare a single volume that was concise in nature but combined the essential components of underground structures. Nature is always smarter than some of us, and sometimes smarter than all of us. In response to those requests, the book was written for a broad spectrum of readers, ranging from engineers seeking technical guidance to owners and other decision makers hoping to glean a better understanding of alternatives. It included the fundamental concepts and methods of underground structures as well as underground engineering, including rock and soil tunnels, shafts, caverns, diaphragm walls and tunnelling methods.

Research into the development of underground structures and design has been extensive during the past five decades. Authors are tempted to include all of the recent developments in a text book; however, since this text is intended as an introductory text, it stresses the fundamental principles without becoming cluttered with too many details and alternatives. In the text itself, the authors have included cross-references to others chapters as appropriate. It is our hope that this combination will help readers locate information more quickly than a traditional, keyword-based index would.

Underground structures is abundantly illustrated to help students understand the material. Several examples are presented in related chapters. Problems are provided at the end of each chapter for homework assignment, and they are all SI units.

The authors have obviously devoted considerable effort to chapter preparation and they thank the various colleagues, publishers, websites and institutions who gave us permission to use and reproduce their copyrighted materials. Texts, tables and figures taken in whole or in part from various sources are acknowledged where they occur in the text.

The authors are especially indebted to Prof. Evert Hoek, Prof. M. Karakus, Specialist R. S. Sinha, Mr. R. J. Fowell, Prof. K. M. Lee, Prof. Xueyuan Hou, Dr. Yiming Sun, Prof. V. Gabriel Guzman and Prof. R. Kastner for their invaluable assistance and permission in preparing and revising the manuscript. Figures and tables from journals, proceedings, and books are reproduced with permission from the respective publishers.

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Activity is the only road to knowledge, we are slow walkers, but we never walk backwards.

Lixin Zhang

Xinyu Hu

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Symbols

A (m^2) = area

A = porewater-pressure coefficient

A_c = activity = I_p/CF

B (m) = width

C (any dimension) = constant

C (kN) = resultant cohesion

CF = clay fraction

CRP = constant rate of penetration

CRS = constant rate of strain oedometer test

c (kPa) = cohesion intercept

c (m/s) = compression wave velocity

c' (kPa) = cohesion intercept of Mohr-Coulomb failure envelope in terms of effective stress at failure

c (m) = constant in Engineering News formula

c_v (m^2/year) = coefficient of consolidation

c' (kPa) = soil cohesion

c_u (kPa) = undrained shear strength

D = discharge factor (in relation to vertical drains)

D (m) = depth; diameter; spacing between centers of piles

D (mm) = grain size

D_{10} (mm) = effective grain size

D_{50} (mm) = mean grain size

D_e (mm) = effective grain size

D_f (m) = depth of foundation

D_r = relative density of cohesionless soil

DS = direct shear test

DSS = direct simple shear test

d (m) = distance

d (mm) = grain size

d_b (m) = base diameter of drilled shaft

d_s (m) = shaft diameter of drilled shaft

E (kPa) = modulus of elasticity

E (kN/m) = normal force on side of slice (stability analysis)

E_i (kPa) = initial tangent Young's modulus

E_s (kPa) = modulus of deformation computed from pressuremeter measurement

E_u (kPa) = undrained Young's modulus

EA (kN) = Normal stiffness of pile or wall

EI ($\text{kN} \cdot \text{m}^2$) = flexural stiffness of pile or wall
 E (kPa) = Young's modulus of tunnel lining (ML-IT-2)
 $(EI)_e$ ($\text{kN} \cdot \text{m}^2/\text{m}$) = effective bending rigidity per unit length of a segmental tunnel
 EI ($\text{kN} \cdot \text{m}^2/\text{m}$) = bending rigidity per unit length of the actual tunnel lining
 ESSA = effective stress stability analysis
 e = void ratio
 e_o = initial void ratio
 F (kN) = reaction; resultant force
 F = factor of safety
 FS = factor of safety
 f = coefficient of friction between soil and base of structure
 f_k (kPa) = physical index of rock mass
 G = shear modulus; also, specific gravity
 $\text{GPa} = 10^3 \text{MPa}$
 g (gal) = acceleration due to gravity
 H (m) = thickness of stratum
 h (m) = thickness of the soil layer
 h_i (m) = thickness of the soil layer i
 IL = incremental loading oedometer test
 I = Moment of inertia of area
 I_L = liquidity index
 I_p = plasticity index
 I_z = vertical strain influence factor
 I_1, I_2, I_3 = invariants of stress
 I (m^4/m) = moment of inertia per unit length of tunnel lining
 I_e (m^4/m) = effective moment of inertia per unit length of tunnel lining
 I_j (m^4/m) = moment of inertia per unit length at the joint
 I_L = liquidity index
 i = hydraulic gradient
 i = angle of the leading edge of an asperity on a joint
 i_p (kN/m^3) = pressure gradient
 J_n = joint set number
 J_r = joint roughness number
 J_a = joint alteration number
 J_w = joint water reduction factor
 K = ratio between intensities of effective horizontal and effective vertical pressures at a given point in a mass of soil
 K_0 = coefficient of earth pressure at rest (value of K for initial state of equilibrium)

K_{op} = coefficient of earth pressure at rest in normally consolidated young deposits
 K_a = coefficient of active earth pressure
 K_p = coefficient of passive earth pressure
 K_{ps} = coefficient of passive earth pressure during passive shearing
 K (kPa) = bulk modulus of soil skeleton
 K (kPa/m) = spring stiffness
 K_h (kPa/m) = modulus of horizontal subgrade reaction
 K_s (kPa/m) = coefficient of soil resistanc; coefficient of subgrade reaction
 K = coefficient of earth pressure
 K_0 = coefficient of earth pressure at rest
 K_s (kN/m³) = soil resistance coefficient
 K_θ (kN · m/(rad · m)) = flexural stiffness of a joint per unit length
 k (m/s) = coefficient of permeability
 l (m) = length
 l (m) = calculation length of a tunnel (usually adopted as 1 m)
 M_c (kN/m) = moment of cohesive forces
 M (kN · m/m) = bending moment per unit length in tunnel lining
 M_p (kN · m/m) = bending moment per unit length due to surrounding earth pressures
 \overline{M}_{pj} (kN · m/m) = bending moment per unit length under the j th loading case
 \overline{M} = internal bending moment per unit length due to a dimensionless unit force
MPa = megapascals (MN/m²)
 N = dimensionless factor
 N_b = base stability number
 N_c = equivalent significant number of uniform shearstress pulses produced by earthquake or imposed in the laboratory
 n = porosity
 n_a = ratio between distance from bottom of lateral support to point of application of earth pressure and total height of lateral support
 n_d = depth factor (stability of slopes)
 n_h (kPa/m) = coefficient of horizontal subgrade reaction
 n_o = initial porosity
 n = number of joints in the tunnel half ring
 N (kN/m) = axial force per unit length in the tunnel lining
 \overline{N} = axial force per unit length due to a dimensionless unit force
 N_p (kN/m) = axial force per unit length due to surrounding earth pressures
 N_{pj} (kN/m) = axial force per unit length under the j th loading case
OCR = overconsolidation ratio = $\sigma'_p/\sigma'_{vo} = \sigma'_{vmax}/\sigma'_{vo}$
 P = percent of grains smaller than given size

P (kN or kN/m) = resultant pressure, normal force
 P_A (kN/m) = active earth pressure if arching is absent (retaining wall; active Rankine state)
 PCPT = push cone penetration test
 P_I = plasticity index
 p_1 (kPa) = vertical overburden earth pressure at the tunnel crown
 p_2 (kPa) = reaction pressure at the bottom of tunnel lining
 p_3 (kPa) = total lateral earth pressure at the crown level of tunnel lining
 p_4 (kPa) = additional earth pressure developed at the tunnel invert level
 p_5 (kPa) = self-weight of the tunnel lining
 p_6 (kPa) = lateral soil resistance pressure
 p (kPa) = surrounding earth pressure around a tunnel
 p_h (kPa) = value of p along the horizontal axis of the tunnel
 p_v (kPa) = value of p along the vertical axis of the tunnel
 p_r (kPa) = soil resistance developed at the springline of a tunnel
 P (kN/m) = applied vertical load (MT-2)
 P_a (kN/m) = active earth pressure if arching is present (bracing in open cuts)
 P_0 (kN) = Surcharge
 P_p (kN/m) = passive earth pressure (May be subdivided into $[P_p]_{II}$, which depends on cohesion and surcharge.)
 P_s (kN) = resultant of forces having seat on surface of particle
 P_w (kN/m) = resultant water pressure
 p (kPa) = pressure of normal stress; subgrade reaction
 p' (kPa) = effective stress = $(\sigma'_1 + \sigma'_3)/2$
 p_A (kPa) = intensity of active earth pressure
 p_L (kPa) = limiting pressure reached in pressuremeter
 p_q (kPa) = increase in pressure on retaining wall due to surcharge q per unit of area
 p_{q1} (kN/m) = increase in pressure on retaining wall due to surcharge q_1 per unit of length parallel to crest
 p_s (kPa) = swelling pressure
 p_1, p_2 (kPa) = secondary principal stresses
 Q (kN) = concentrated load; strut load
 q_1 (kPa) = total earth pressure developed above tunnel level
 q_2 (kPa) = total earth pressure developed at the shoulder regions of a tunnel
 \bar{Q} = shear force per unit length due to a dimensionless unit force
 Q_p (kN/m) = shear force per unit length due to surrounding earth pressures
 Q_{pj} (kN/m) = shear force per unit length under the j th loading case
 q (kPa) = average gross bearing pressure over the foundation
 q (kPa) = shear stress = $(\sigma_1 - \sigma_3)/2$

q (kPa) = uniformly distributed load; surcharge per unit of area; uniformly distributed line load
 q (m³/day) = seepage rate or flow rate
 q_a (kPa) = allowable soil pressure
 q_c (kPa) = the pressure at which the material passes from the elastic into the semiplastic or plastic state
 q_f (kPa) = bearing capacity
 q_t (kPa) = cone tip resistance
 q_u (kPa) = unconfined compressive strength
 R = ratio of size of filter material to size of material to be protected
 R (m) = radius of influence of well; radius of curvature of deformed slope
 R_c = uniaxial compression strength
 R_f = parameter used in hyperbolic stress strain equation
 R_s = total surcharge ratio
 R'_s = effective surcharge ratio
 RMR = rock mass rating according to the Geomechanics Classification
 RQD = Rock Quality Designation
 R (m) = tunnel calculation radius defined as the average of outer (R_o) and the inner (R_i) radii (L)
 R_d = displacement ratio
 R_{hd} = horizontal displacement ratio
 R_{vd} = vertical displacement ratio
 r (m) = radius
 r_o = radius of logarithmic spiral
 S (kN/m) = total sliding resistance between base of dam and subsoil
 S (mm) = settlement; heave; penetration of pile under hammer blow
 S = shear wave
 S = spacing between joints of a given set
 SML = slow maintained load
 SRF = stress reduction factor
 S_c (mm) = settlement at end of construction and application of permanent live load
 S_i (kPa) = shear strength intercept according to the Mohr Coulomb relationship (' cohesion')
 S_j (kPa) = shear strength intercept for a joint
 S_p (mm) = settlement at end-of-primary consolidation
 S_r = degree of saturation
 S_s = degree of sensitivity
 SQD = specimen quality designation
 s (kPa) = shearing resistance; shear strength; drained shear strength
 s_u (kPa) = undrained shear strength
 s_u (critical) (kPa) = undrained shear strength of liquefied sand

$s_u(\text{mob})$ (kPa) = undrained shear strength mobilized in full-scale field failures; undrained shear strength mobilized during stage construction
 $s_{uo}(\text{DSS})$ (kPa) = preconstruction undrained shear strength measured by direct simple shear test
 $s_{uo}(\text{FV})$ (kPa) = preconstruction undrained shear strength measured by field vane test
 $s_{uo}(\text{TC})$ (kPa) = preconstruction undrained shear strength measured by triaxial compression test
 $s_{uo}(\text{TE})$ (kPa) = preconstruction undrained shear strength measured by triaxial extension test
 $s_{uo}(\text{UC})$ (kPa) = preconstruction undrained shear strength measured in unconfined compression test
 $s_{uo}(\text{UU})$ (kPa) = preconstruction undrained shear strength from unconsolidated undrained compression test
 s_{uo} (kPa) = preconstruction undrained shear strength
 T (kN/m) = shear force on side of slice (stability analysis)
 T (degrees centigrade) = temperature
 T_c = construction time factor
 T_{MR} = magnitude of the flexural tensile strength (“modulus of rupture”)
 T_r = time factor for radial consolidation (vertical compression with radial flow)
 T_v = time factor for one-dimensional consolidation in vertical direction
 T_0 = magnitude of the tensile strength; uniaxial tensile strength unless indicated other wise
 TC = triaxial compression test
 TE = triaxial extension test
 t (s) = time t (m) = thickness of the tunnel lining
 t (m) = thickness of the tunnel lining
 t_c (days) = construction time
 U = average degree of consolidation
 UU = unconsolidated undrained triaxial compression test
 $USSA$ = undrained strength stability analysis
 u (kPa) = porewater pressure
 u, v (m) = displacements parallel to x, y ; positive in positive direction of coordinate axis
 u_r, u_θ = displacements parallel to r, θ
 Δu (mm) = shear displacement along a joint; also radial deformation
 Δh (mm) = the horizontal displacement of the tunnel lining at the springline level
 Δv (mm) = normal displacement across a joint
 Δ'_h (m) = predicted horizontal displacement of the equivalent tunnel by matching the vertical displacement in the equivalent process
 Δ_p (m) = displacement developed at the location of the redundant force x_i due to the action of surrounding earth pressures
 Δ_v (m) = vertical displacement of a tunnel defined as the total change of the tunnel diameter in the vertical direction
 Δ'_v (m) = predicted vertical displacement of the equivalent tunnel by matching the horizontal displacement in the equivalent process

V (m³) = total volume
 ν_s = Poisson's ratio of soil
 W (kN or kN/m) = weight
 w = water content in percent of dry weight
 w_l = liquid limit
 w_o = natural water content in percent of dry weight
 w_p = plastic limit
 w_s = shrinkage limit
 x, y, z = right-handed Cartesian coordinates
 z (m) = depth, position, or elevation head
 z_c (m) = depth of tension cracks
 α = angle
 α = pressuremeter rheological coefficient
 α = reduction factor on strength of clay adjacent to shaft of pier
 β (degrees) = slope angle
 γ (kN/m³) = total unit weight
 γ' (kN/m³) = submerged unit weight
 γ_c (kN/m³) = average unit weight of the concrete (lining material, etc.)
 γ_d (kN/m³) = dry unit weight, unit weight of soil if water is entirely replaced by air
 γ_w (kN/m³) = unit weight of water
 γ_s (kN/m³) = average unit weight of solid constituents
 γ_{sat} (kN/m³) = unit weight in saturated state
 γ_i (kN/m³) = total unit weight of soil layer i
 γ_{as} (kN/m³) = average total unit weight of soil at the shoulder regions
 γ_c (kN/m³) = unit weight of lining material
 γ_w (kN/m³) = unit weight of water
 Δ = change
 δ (degrees) = angle of wall friction; angle between resultant stress on plane and normal to plane
 δ (mm) = displacement at a point of rock mass
 δ_h (mm) = lateral movement
 δ_{hmax} (mm) = maximum lateral movement
 δ_{ii} = displacement developed at the location of the redundant force x_i and displacing in the same direction as x_i due to the action of a dimensionless unit force $x_i = 1$
 δ_{ij} = displacement developed at the location of the redundant force x_i and along the direction of x_i due to the action of a dimensionless unit force $x_j = 1$
 ε = base of Napierian logarithms; unit strain
 ε_c = compression strain index
 ε_f = axial strain at failure

ε_R = reference strain

ε_v = vertical strain

ε_α = secondary compression strain index

η (kN/m²) = effective bending rigidity ratio, viscosity

θ (degrees) = angle; central angle

λ = joint stiffness ratio; coefficient of the lateral pressure; creep parameter

μ = Poisson's ratio; field vane correction factor for embankment and foundation stability analysis; micron; friction coefficient (= tan ϕ); also same as η

μ_{UC} = correction factor for unconfined compression test on specimens from D to B quality

μ_s = field vane correction factor for slope stability analysis

μ_t = time-to-failure correction factor for undrained shear strength from laboratory tests

ν = Poisson's ratio

ρ = mass density (FL⁻⁴T²)

ρ_d (Mg/m³) = density in dry state

ρ_s (Mg/m³) = average density of solid constituents

ρ_{sat} (Mg/m³) = density in saturated state

ρ_w (Mg/m³) = density of water

σ (kPa) = total normal stress, normal stress at the cross section of the tunnel lining

σ_g (kPa) = normal grout stress

σ' (kPa) = effective normal stress

σ_1 (kPa) = major principal stress

σ_2 (kPa) = intermediate principal stress

σ_3 (kPa) = minor principal stress

σ'_1 (kPa) = effective major principal stress

σ'_2 (kPa) = effective intermediate principal stress

σ'_3 (kPa) = effective minor principal stress

σ'_c (kPa) = preconsolidation pressure or critical pressure resulting from secondary compression

σ'_{1c} (kPa) = effective major principal stress after consolidation and before undrained shear

σ'_{3c} (kPa) = effective minor principal stress after consolidation and before undrained shear

$\sigma'_{\alpha\alpha}$ (kPa) = in situ effective normal stress on a plane with orientation α to the horizontal

σ'_h (kPa) = effective horizontal pressure

σ'_n (kPa) = effective normal stress on a plane

σ'_p (kPa) = preconsolidation pressure

σ'_{pl} (kPa) = preconsolidation pressure measured in isotropic consolidation test

σ'_s (kPa) = isotropic effective stress in an undisturbed specimen

σ'_v (kPa) = total vertical stress

σ'_{vc} (kPa) = vertical consolidation pressure

σ'_{ver} (kPa) = critical pressure at which the bearing plate on unsaturated soils with metastable struc-

ture plunged into the ground

σ_{vf} (kPa) = postconstruction total vertical stress

σ'_{vf} (kPa) = final effective vertical stress = $\sigma'_{vo} + \Delta\sigma'_v$

σ'_{max} (kPa) = maximum past pressure

σ_{vo} (kPa) = preconstruction total vertical stress

σ'_{vo} (kPa) = in situ effective vertical stress, effective overburden pressure

σ'_{vs} (kPa) = maximum effective vertical stress reached before the removal of surcharge

$\Delta\sigma'_a$ (kPa) = axial stress imposed during the second step of a triaxial test

τ (kPa) = shear stress

τ (seismic) (kPa) = equivalent uniform shear stress pulse produced by an earthquake

τ_c (kPa) = shear stress carried by soil after consolidation and before undrained shear

τ_h (kPa) = shear stress on horizontal planes

τ_{max} (kPa) = maximum shear stress pulse produced by an earthquake

$\tau_{r\theta}$ (kPa) = the shear stress in the radial of tangential direction, at any point of ground

τ_s (kPa) = shear stress applied under undrained conditions followed by consolidation

τ_v (kPa) = shear stress on vertical planes

τ_{α} (kPa) = in situ shear stress on a plane with orientation α to the horizontal

Φ = velocity potential (flow net)

ϕ' (degree) = effective-stress friction angle; effective internal friction angle of soil; angle of shearing resistance

ϕ'_{cv} (degree) = constant-volume friction angle

ϕ'_d (degree) = component of friction angle produced by dilation

ϕ'_g (degree) = component of friction angle of granular soils resulting from geometrical interference

ϕ'_j = friction angle for a joint

ϕ'_m (degree) = effective-stress friction angle mobilized at undrained yield condition

ϕ'_p (degree) = component of friction angle of granular soils resulting from particle rearrangement

ϕ'_s (degree) = secant friction angle for granular soils

ϕ'_μ (degree) = angle of interparticle sliding friction

φ (degree) = internal friction angle of soil/rock

ψ (degrees) = angle of dilatancy

Ψ = function defining flow lines; angle between the direction of σ_1 and the plane of a joint

$\bar{\omega}$ = average displacement of a bearing plate

ω (%) = moisture content of the soil

$\ln a$ = Napierian (natural) logarithm of a

$\log a$ = logarithm of a to the base 10

x_1 (kN · m/m) = bending moment per unit length acting at the tunnel crown

x_2 (kN · m/m) = axial force per unit length acting at the tunnel crown

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