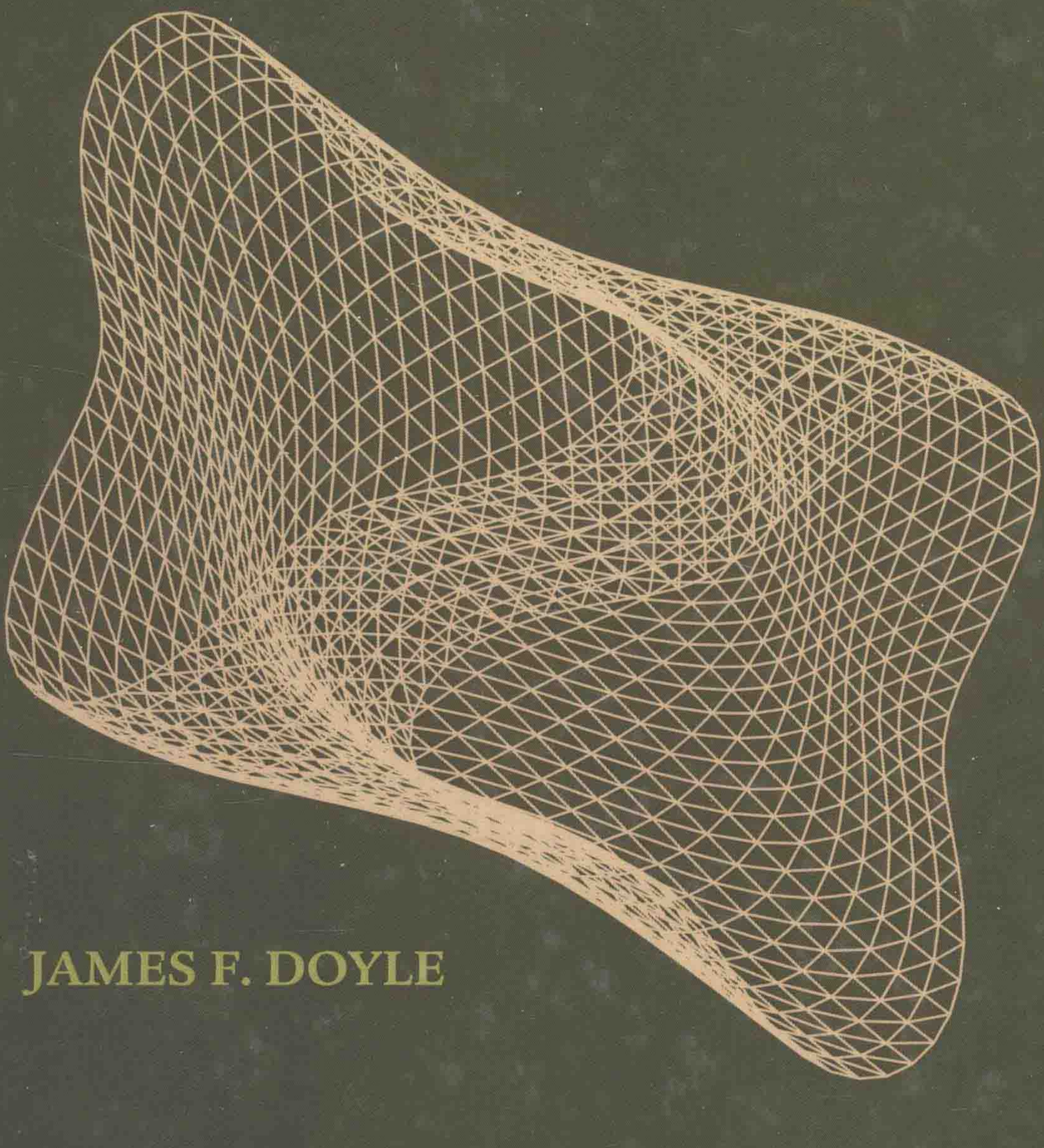
Nonlinear Structural Dynamics Using FE Methods



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Notation

Roman letters:

a	radius, plate width
A	cross-sectional area
b, b_i	thickness, depth, plate length, body force
c_o	longitudinal wave speed, $\sqrt{E/\rho}$
c_P, c_S, c_R	primary $= \sqrt{E^*/\rho}$, secondary $= \sqrt{G/\rho}$, and Rayleigh wave speeds
$C, [\ C\ ]$	damping, damping matrix
D	plate flexural rigidity, $Eh^3/12(1-\nu^2)$
$\hat{e}_i$	unit vectors
$E, \hat{E}, E^*, \bar{E},$	Young's modulus, $E^* = E/(1-\nu^2)$, $\bar{E} = E^*h$
EI	beam flexural stiffness
E_{ij}	Lagrangian large-strain tensor
$F, \hat{F}, \bar{F}_o, F_i$	member axial force, element nodal force
$\mathcal{F}$	generalized nodal force
$g_i(x)$	element shape functions
$G, \hat{G}$	shear modulus, frequency-response function
h	beam or rod height, plate thickness
h_i	interpolation functions
i	complex $\sqrt{-1}$, counter
I	second moment of area, $I = bh^3/12$ for rectangle
J^o, J, J_e	Jacobian
J_n	Bessel functions of the first kind
k, k_1, k_2	wavenumbers
$K, [\ k\ ], [\ K\ ]$	stiffness, stiffness matrices
L	length
M, M_x	moment
$M, [\ m\ ], [\ M\ ]$	mass, mass matrices
$P(t), \hat{P}, \{P\}$	applied-force history
$\mathcal{P}$	generalized applied load
q, q_u, q_v, q_w	distributed load

r, R	radial coordinate, radius
r, s, t	isoparametric coordinates
$[\ R\ ]$	rotation matrix
t, t_i	time, traction vector
T	time window, period, temperature
$\mathcal{T}$	kinetic energy
$[\ T\ ]$	transformation matrix
$u(t)$	response; velocity, strain, etc.
u, v, w	displacements
$\mathcal{U}$	strain energy
V	member shear force, volume
$\mathcal{V}$	potential of conservative loads
W	beam width, Wronskein
$\mathcal{W}$	work
x^o, y^o, z^o	original rectilinear coordinates
x, y, z	deformed rectilinear coordinates

Greek letters:

α	coefficient of thermal expansion
δ	small quantity, variation
δ_{ij}	Kronecker delta
Δ	determinant, increment
ϵ, ϵ_{ij}	small quantity, strain
η	viscosity, damping, principal coordinate
θ	angular coordinate
λ	eigenvalue
μ	shear modulus, complex frequency
ν	Poisson's ratio
Π	total potential energy
ρ^o, ρ	mass density
σ, σ_{ij}	stress
ϕ	phase
ϕ_x, ϕ_y, ϕ_z	rotation
$\{\phi\}, [\ \Phi\ ]$	modal vector, matrix
ω	angular frequency
ω_o, ω_d	natural frequency, damped natural frequency
ξ	damping ratio

Special symbols:

∇^2	differential operator, $(\partial^2/\partial x^2)+(\partial^2/\partial y^2)$
$[\quad]$	square matrix, rectangular array
$\{\ \}$	vector, spectral amplitude
$\lceil\quad\rfloor$	diagonal matrix
$\bar{\ }$	(bar) local coordinates

$\hat{}$	(hat) vector, complex quantity
$\dot{}$	(dot) time derivative

Subscripts:

E, G, T	elastic, geometric, tangent (total) stiffness matrix
i, j, k	continuum tensor components
,	(comma) partial differentiation

Superscripts:

o	original configuration
$*$	complex conjugate
$'$	prime, derivative with respect to argument

Abbreviations:

BC, PBC	boundary condition, periodic BC
DoF, SDoF	degree of freedom, single DoF
DKT	discrete Kirchhoff triangular FE element
EoM	equation of motion
EVP	eigenvalue problem
FE	finite element
FFT	fast Fourier transform
FRF	frequency-response function
IC	initial condition
MRT	membrane with rotation triangular FE element
ODE	ordinary differential equation

Primary Examples of Notation Use

Discrete Systems:

$$\frac{\partial \mathcal{U}}{\partial u_I} - \mathcal{P}_I = 0 \qquad I = 1, 2, \ldots, N$$

where $\mathcal{U}$ is strain energy, u_I is generalized DoF, $\mathcal{P}_I$ is generalized force, N is total number of DoF, and I is the enumeration of DoF.

Continuum Systems:

$$\epsilon_{ij} = \frac{\partial u_i}{\partial x_j^o} \qquad i,j = 1, 2, 3$$

where ϵ_{ij} is strain tensor, u_i is Cartesian strain component, x_i^o is Cartesian position component, and i, j is Cartesian tensor component.

Atomic Systems:

$$P_i^\alpha = -\frac{\partial \mathcal{V}}{\partial u_i^\alpha} \qquad \alpha = 1, 2, \ldots, N, \quad i = 1, 2, 3$$

where P_i^α is Cartesian force component, $\mathcal{V}$ is load potential, u_i is Cartesian displacement, i is Cartesian tensor component, N is total number of atoms, and α is the enumeration of atoms.

Atomic EoM:

$$M^{\alpha}\frac{d^2\hat{r}^{\alpha}}{dt^2}=\sum_{\beta\neq\alpha}^{N}\hat{P}^{\alpha\beta}\qquad \hat{r}=\sum\nolimits_{i}^{3}x_i\hat{e}_i=\text{vector}$$

where M^{α} is mass, $\hat{r}^{\alpha}$ is position vector, $\hat{P}^{\alpha\beta}$ is force vector on atom α due to atom β, and N is the total number of atoms.

Contents

Introduction

> Those who have meditated on the beauty and utility of the general method of Lagrange – who have felt the power and dignity of that central dynamical theorem which he deduced from a combination of the principle of virtual velocities with the principle of D'Alembert – and who have appreciated the simplicity and harmony which he introduced by the idea of the variation of parameters, must feel the unfolding of a central idea.
>
> *W. R. Hamilton [41]*

Structures are to be found in various shapes and sizes for various purposes and uses. These range from the human-made structures of bridges carrying traffic, buildings housing offices, and airplanes carrying passengers all the way down to the biologic structures of cells and proteins carrying genetic information. Structural mechanics is concerned with the behavior of structures under the action of applied loads – their deformations and internal loads. We present, in the following chapters, versatile methods to tackle some of the most common (and most difficult) problems facing engineers in the analysis of structures. This volume specifically considers the situations where the loads vary in time such that inertia effects are important in computing the responses.

The modeling of the dynamic response of structures introduces many additional considerations not anticipated from a static analysis. It is therefore worth our while to say just a little about *structural dynamics* and its place in *structural analyses*. The subject of rigid-body dynamics treats physical objects as bodies that undergo motion without any change of shape. This has many applications: the movement of machine parts, the flight of an aircraft or space vehicle, the motion of the Earth and the planets. In many instances, however, the primary concern is dynamic response involving changes of shape. This is particularly so in the design of structures as encountered in automobiles, ships, aircraft, space vehicles, offshore platforms, buildings, and bridges. Dynamic response involving deformations is usually oscillatory in nature; the structure vibrates about a configuration of stable equilibrium. For example, suppose that a building structure is in a state of static equilibrium under the gravity loads acting on it; when subjected to wind loading, the structure oscillates about this position of static equilibrium. An airplane provides an example of oscillatory motion about an equilibrium configuration that involves rigid-body motion. When in flight, the whole system moves as a rigid body but is also subjected to oscillatory motion due to engine and aerodynamic loads.

With the increasing use being made of lightweight, high-strength materials, structures today are more susceptible than ever before to critical vibrations. Modern buildings and bridges are lighter and more flexible and are made of materials that provide much lower energy dissipation; all these contribute to more intense vibration responses. Dynamic analysis of structures is therefore important for modern structures and is likely to become even more so.

Structural Analysis and Models

The term *model* is widely used in many different contexts, but here we mean a representation of a physical system that may be used to predict the behavior of a system in some desired respect. The actual physical system for which the predictions are to be made is called the *prototype*.

There are two broad classes of models: physical models and mathematical models. The physical model resembles the prototype in appearance but is usually of a different size, may involve different materials, and frequently operates under loads, temperatures, and so on that differ from those of the prototype. The use of these models belongs to the category of "experimental methods of structural analysis." The mathematical model consists of one or more equations (and, more likely nowadays, a numerical finite-element model) that describe the behavior of the system of interest. The equations of the model are based on certain basic laws and principles of mechanics and usually involve simplifying assumptions. These models broadly belong to the category of "analytical methods of structural analysis" (which sounds a bit tautological). The equations themselves may or may not be solved on a computer.

With the development of a valid model, it is possible to predict the immediate and future behavior of the prototype under a set of specified inputs and to examine a priori the effect of various possible design modifications.

We deal with models of different types in this book. Our primary model for "solving the problem" is finite-element (FE)–based and represents the structure in terms of a finite number of discrete unknowns. This approach is chosen because it is easily implemented on a computer and is scalable to large systems. It is also true that current commercial FE codes are such that once the geometry, material properties, and so on are correctly specified, then very high quality models are produced that give high quality predictive capability, and this must inform how structural dynamics should now be done.

Two practical finite elements are emphasized: the frame FE and the solid FE. These represent extremes in a way: the frame FE embeds many structural assumptions about behavior of slender members and consequently is very efficient where applicable, and the solid FE has no structural assumptions and hence is applicable to frames, shells, and solids alike, but it can be computationally expensive to use. With these two element models available, almost any structural problem can be solved in the sense that given the geometry, material properties, loads, and so on, responses can be generated. This is where a different level of model enters, one that helps to explain the computed numbers; these are of the "simple model" type. That is, when trying to understand a complex system, it is quite useful (and arguably necessary) to have available these simple models – not as solutions per se but as organizational principles for seeing through the voluminous numbers produced by the FE codes. They identify the model parameters that play a significant role. Sometimes they are constructed to go deeper into the mechanics of a problem; for example, there are the plate and shell simple models that follow the structural consequence of the thickness being thin, but there are also the modal, spectral, and wave-propagation *analysis models* that give insight into how to view and understand structural dynamics.

Goals and Outline of the Book

The primary goals of this book are

- To develop solution methods, general enough and scalable enough, to solve "big dynamics problems"
- To develop methods of analysis to "make sense" of the generated solutions
- Because geometric nonlinearities are an intimate aspect of flexible structures, to make the solution and analysis methods general enough to seamlessly handle nonlinear problems

The book is divided into two parts roughly corresponding to the first two objectives. Part I develops the mechanics and computer models to handle general problems, Part II develops the analytical models. The nonlinear analyses are distributed throughout the chapters.

Chapter 1 introduces some foundational ideas in the dynamics of elastic systems, ideas such as resonance and damping. Chapter 2 develops the mechanics needed to handle large, complex systems; the key concept introduced is that of virtual work. The formulation is in terms of discretized systems and is general enough to be applicable to static/dynamic, linear/nonlinear, and conservative/nonconservative systems alike. The only restriction is that the system be discretized. Chapter 3 uses the Ritz method to convert continuous systems into discrete form and then formalizes the process via the FE method. Chapter 4 is an attempt to classify the various types of dynamic problems based on the space-time variation of their loadings; this sets the contexts for the computational tools required and for the types of analysis procedures introduced. Chapter 5 ends Part I with a review of some of the computer methods used to implement our models. The essential algorithms discussed in detail are those for time integration of simultaneous equations and for solving eigenvalue problems.

Part II is the compilation of analytical models: modal analysis and eigenvalue problems in Chapter 6, spectral analysis and strong formulations in Chapter 7, flexible plates and shells in Chapter 8, and wave propagation and high-frequency analysis in Chapter 9. Chapter 10 ends Part II with an introduction to the concept of the stability of the motion. This is fundamentally a nonlinear notion, and the preceding chapters are developed general enough to anticipate this.

There are a good number of example problems distributed throughout the chapters. A few are straightforward "finger exercises" in that they take a previously established result and apply it to some problem. A few others do direct extensions of some developed model or result. A very exciting new type of example problem is made available because of Chapters 3 and 5 and is the predominant type of example problem in Part II; here the computer programs are used to produce results, but the analysis challenge is to "explain" the results. This is akin to the experimental challenge of collecting data on some partially unfamiliar or unknown dynamic problem and then trying to explain the data. The objective of the example problem is not so much to explain this or that result per se, but to show how the programs can be interacted with to produce additional data to aid discovering the explanation of the results. Remember that unlike the experimental analogue, the FE solution can provide almost unlimited information about the solution presented in almost unlimited different forms. Therefore, having control of the postprocessing capabilities is an important aspect of the analysis.

In terms of philosophy, any FE program can be used for the underlying computations, but to affirm the integral aspect of the computations and the analyses, source code and executables are provided on the accompanying website: www.cambridge.org/doyle_structures_FEM. These codes and executables can be used to re-create the data used in the majority of the examples as well as extend them. The two major codes are `SDframe`/`SDsolid`; these are leaner versions of the programs used in the `QED` package [28]. Also, as additional encouragement to reproduce the results, all example problems are documented in terms of dimensions, material properties, boundary conditions, and loadings, as well as mesh information.

As a final point, structural analysis computer programs generally do not use a built-in system of units and do not utilize any dimensional conversion constants. Therefore, any consistent system of units may be used for input, and the corresponding calculated results are output in the same units. All the relevant data for the example problems are presented in both SI units and common units. Because both systems of units are used in this book, we generally prefer (when convenient) to present the results in nondimensional form; however, for units that are common to both systems (e.g., time, frequency, angle), they are left as is.

PART I

MECHANICS AND MODELS

1 Dynamics of Simple Elastic Systems

This chapter is concerned with the formulation of the equations of motion (EoM) of simple systems. What is meant by *simple* is that the systems have just a single degree of freedom (SDoF) and does not imply that the underlying mechanics is simple or elementary in any way.

The concept of vibration is fundamental to understanding the dynamics of elastic structures. The study of vibration is concerned with the oscillatory motion of bodies; all bodies with elasticity and mass are capable of exhibiting vibrations. Resonant (or natural) frequencies are the frequencies at which a structure exhibits relatively large response amplitudes for relatively small inputs. Even if the excitation forces are not sinusoidal, these frequencies tend to dominate the response. In practice, large resonant responses are mitigated by the presence of damping and nonlinear effects. Damping is considered in this chapter, whereas the effects of nonlinearities are distributed throughout the other chapters. The use of Fourier analysis (or spectral analysis) as a means of describing time-varying behavior is essential to the study of structural dynamics, and this too is developed in this chapter.

1.1 Motion of Simple Systems

This section reviews the dynamics of elastic systems in the form of a spring-mass-dashpot. We restrict the emphasis to concepts that are used directly in this and later chapters. References 45, 81, and 83 are good sources for additional details on the material covered here.

Newton's Laws for Moving Masses

Everyday experience of mass is as a weight, so much so that the words *mass* and *weight* are often used interchangeably. In dynamics, these two words are associated with quite distinct concepts – the first with inertia and the second with gravitation attraction. We elaborate on the difference between the two.

Consider a particle as an object with negligible dimensions but definite mass M and definite position in three-dimensional (3D) space $\hat{r} = x\hat{\imath} + y\hat{\jmath} + z\hat{k}$, where $\hat{\imath}$, $\hat{\jmath}$,