SOIL MECHANICS

CALCULATIONS PRINCIPLES AND METHODS

Victor N. Kaliakin



Soil Mechanics

Calculations, Principles, and Methods

Victor N. Kaliakin University of Delaware, Newark, DE





Butterworth-Heinemann is an imprint of Elsevier The Boulevard, Langford Lane, Kidlington, Oxford OX5 1GB, United Kingdom 50 Hampshire Street, 5th Floor, Cambridge, MA 02139, United States

Copyright © 2017 Elsevier Inc. All rights reserved.

No part of this publication may be reproduced or transmitted in any form or by any means, electronic or mechanical, including photocopying, recording, or any information storage and retrieval system, without permission in writing from the publisher. Details on how to seek permission, further information about the Publisher's permissions policies and our arrangements with organizations such as the Copyright Clearance Center and the Copyright Licensing Agency, can be found at our website: www.elsevier.com/permissions.

This book and the individual contributions contained in it are protected under copyright by the Publisher (other than as may be noted herein).

Notices

Knowledge and best practice in this field are constantly changing. As new research and experience broaden our understanding, changes in research methods, professional practices, or medical treatment may become necessary.

Practitioners and researchers must always rely on their own experience and knowledge in evaluating and using any information, methods, compounds, or experiments described herein. In using such information or methods they should be mindful of their own safety and the safety of others, including parties for whom they have a professional responsibility.

To the fullest extent of the law, neither the Publisher nor the authors, contributors, or editors, assume any liability for any injury and/or damage to persons or property as a matter of products liability, negligence or otherwise, or from any use or operation of any methods, products, instructions, or ideas contained in the material herein.

Library of Congress Cataloging-in-Publication Data

A catalog record for this book is available from the Library of Congress

British Library Cataloguing-in-Publication Data

A catalogue record for this book is available from the British Library

ISBN: 978-0-12-804491-9

For information on all Butterworth-Heinemann publications visit our website at https://www.elsevier.com/books-and-journals





Working together to grow libraries in developing countries

www.elsevier.com • www.bookaid.org

Publisher: Joe Hayton

Acquisition Editor: Ken McCombs Editorial Project Manager: Peter Jardim

Production Project Manager: Mohanapriyan Rajendran

Designer: Maria Ines Cruz

Typeset by TNQ Books and Journals

Preface

The first course in soil mechanics typically proves to be challenging for undergraduate students. This is due to the fact that soils are three-phase particulate materials, and thus must be treated differently than other engineering materials that undergraduates are introduced to as part of their curriculum. The situation is further complicated by the need to account for the presence of pore fluid, both under hydrostatic and transient conditions, as well as the subject of shear strength.

One of the biggest difficulties in teaching soil mechanics is the lack of lecture time in which to present a sufficient number of example problems, with varying degrees of difficulty, that illustrate the concepts associated with the subject. This book has been written to address the aforementioned shortcoming. It presents worked example problems that will facilitate a student's understanding of topics presented in lecture. This book is not meant to replace existing soil mechanics textbooks but to serve as a supplementary resource.

Victor N. Kaliakin

Acknowledgments

Professor Namunu (Jay) Meegoda from the New Jersey Institute of Technology first suggested the idea for the present book and encouraged me to undertake the task of writing it. The example problems presented in the book have been developed over several years of teaching soil mechanics. Some of the more challenging problems are patterned after similar ones that were provided by my former University of Delaware colleague, Dr. Dov Leshchinsky. Finally, special thanks goes to my current colleague, Dr. Kalehiwot Nega Manahiloh for critically reviewing select chapters of the book and for providing some ideas for example problems.

Cheers, Victor N. Kaliakin

Contents

Prefac Ackno	e wledgn	nents		xi xiii	
1.	Example Problems Involving Phase Relations for Soils				
	1.0		General Comments		
	1.1	Genera	al Definitions	1	
	1.2	Mass D	Densities	3	
	1.3	Unit W	/eights	4	
	1.4		ion of Fundamental Quantities	1 3 4 4 5	
	1.5	Relations Derived From Fundamental Quantities		5	
		1.5.1	Case 1.1: Relation Between Void Ratio and Porosity	.5	
		1.5.2	4	5	
		1.5.3	Case 1.3: Relation Between Moisture Content,		
			Specific Gravity of Solids, Void Ratio, and Degree		
			of Saturation	6	
		1.5.4	Case 1.4: Relation Between Dry Unit Weight,		
			Specific Gravity of Solids, and Void Ratio	6	
		1.5.5	Case 1.5: Relation Between Moist Unit Weight,		
			Specific Gravity of Solids, Moisture Content,		
			and Void Ratio	7	
		1.5.6	Case 1.6: Relation Between Moist Unit Weight,		
			Dry Unit Weight, and Moisture Content	7	
		1.5.7	Case 1.7: Relation Between Moist Unit Weight,		
			Specific Gravity of Solids, Degree of Saturation,		
			and Void Ratio	7	
		1.5.8	Case 1.8: Unit Weight of Submerged Soil and Its		
			Relation to Moist Unit Weight	7	
		Examp	le Problems	8	
2.	Exan	nole Pr	oblems Related to Soil Identification		
		Classifi		51	
	2.0		al Comments	51	
	2.1			51	
	2.2		ution of Grain Sizes	54	
		2.2.1	Sieve Analysis	54	
		2.2.2	Quantities Computed From Gradation Curves	55	

	2.3 2.4 2.5	2.2.3 Importance of Soil Gradation 2.2.4 Hydrometer Analysis Plasticity of Soil Atterberg Limits 2.4.1 Basic Definitions 2.4.2 Derived Limits Soil Classification Example Problems	56 56 59 61 61 63 65	
3.	Exar	nple Problems Related to Compaction of Soils	93	
	3.0	General Comments	93	
	3.1	Fundamental Definitions	93	
		Example Problems	97	
4.	Stresses, Strains, and Elastic Response of Soils			
	4.0	Introductory Comments	131	
	4.1	General Definitions	131	
		4.1.1 The Continuum Concept	131	
		4.1.2 Homogeneity	132	
		4.1.3 Isotropy	132	
	4.2	Concept of Stress	132	
		4.2.1 Definition of Stress at a Point	132	
		4.2.2 Definition of the State of Stress at a Point	134	
		4.2.3 Mean Stress	134	
		4.2.4 State of Plane Stress	135	
		4.2.5 Stress Transformations	135	
	4.3	Deformation and Strain	150	
		4.3.1 Normal and Shear Strains	152	
		4.3.2 Infinitesimal Strains	152	
		4.3.3 Definition of State of Strain at a Point	153	
		4.3.4 Volumetric Strain	154	
		4.3.5 State of Plane Strain	154	
		4.3.6 Strain Transformations	154	
	4.4	Constitutive Relations	155	
		4.4.1 General Form of Constitutive Relations	156	
		4.4.2 Insight Into the Constitutive Matrices	156	
		4.4.3 General Classes of Material Idealizations	157	
		4.4.4 Elastic Material Idealizations	157	
	4.5	Stresses in Soil Due to Surface Loads	160	
	4.6	Superposition Principle	160	
		Example Problems	160	
5.		mple Problems Involving In Situ Stresses Under rostatic Conditions	205	
	5.0	General Comments	205	
	5.1	Surface Tension	205	
	J. 1	5.1.1 Surface Tension Phenomena	205	

	5.2	Capillary Phenomena in Tubes	206
	5.3	Capillary Phenomena in Soils	209
	5.4	In Situ Stresses in Soils Under Hydrostatic Conditions	211
		5.4.1 Total Stress	211
		5.4.2 Pore Fluid Pressure	212
		5.4.3 Effective (Intergranular) Stress	212
	5.5	Relationship Between Horizontal and Vertical Stresses	213
		Example Problems	214
6.	Exan	nple Problems Involving One-Dimensional	
	Fluic	l Flow in Soils	243
	6.0	General Comments	243
	6.1	Conservation of Mass	244
	6.2	Bernoulli's Energy Equation	245
	6.3	Head Loss	247
	6.4	Hydraulic Gradient	247
	6.5	Seepage Velocity	247
	6.6	Darcy's Law	250
	6.7	Experimental Determination of Permeability	252
		6.7.1 Constant-Head Permeability Test	253
		6.7.2 Falling-Head Permeability Test	254
	6.8	Hydrostatic Conditions Compared to Upward	
		and Downward Seepage	255
		6.8.1 No Seepage (Hydrostatic Conditions)	255
		6.8.2 Downward Seepage	257
		6.8.3 Upward Seepage	258
	6.9	Seepage Forces	260
	6.10	Critical Hydraulic Gradient for Upward Seepage	261
	6.11	One-Dimensional Seepage Through Anisotropic	
		Soil Strata	262
		6.11.1 Equivalent Horizontal Permeability	262
		6.11.2 Equivalent Vertical Permeability	263
		Example Problems	265
7.	Exan	nple Problems Involving Two-Dimensional	
	Fluid	flow in Soils	315
	7.0	General Comments	315
	7.1	Basic Assumptions	316
	7.2	Governing Equation	317
	7.3	Boundary Conditions	317
	7.4	Solution of the Governing Equation	317
	7.5	Flow Nets	318
	7.6	Rate of Flow Through Flow Nets	319
		Evample Problems	321

8.	Example Problems Related to Compressibility and Settlement of Soils		
	8.0	General Comments	331
	8.1	Deformation	331
	8.2	Compressibility of Soils	332
	8.3	Settlement	332
		8.3.1 Immediate Settlement	333
		8.3.2 Primary Consolidation Settlement	333
		8.3.3 Secondary Consolidation Settlement	334
	8.4	Quantifying Soil Compressibility	334
	8.5	Preconsolidation Pressure	339
	8.6	Coefficient of Compressibility	340
	8.7	Ultimate Primary Consolidation Settlement	341
	8.8	Coefficient of Volume Compressibility, Modified	
		Compression, and Swell Indices	342
		Example Problems	344
9.	Evan	unla Problems Polated to Time Pate	
9.	Example Problems Related to Time Rate of Consolidation		
			377
	9.0	General Comments	377
	9.1	Fundamental Definitions	377
	9.2	Terzaghi's One-Dimensional Consolidation Theory	377
		9.2.1 Governing Differential Equation	379
		9.2.2 Separation of Variables Solution	38
		9.2.3 Local Degree of Consolidation	38
		9.2.4 Average Degree of Consolidation	382
		Example Problems	384
10.	Example Problems Related to Shear Strength		
	of So	ils	419
	10.0	General Comments	419
	10.1	Shear Strength of Soils	419
	10.2	Factors Controlling Shear Strength of Soils	419
	10.3	Volume Change Characteristics	420
	10.4	Importance of Shear Strength of Geomaterials	420
	10.5	Mohr's Failure Criterion	420
	10.6	Mohr-Coulomb Failure Criterion	422
		10.6.1 Obliquity Relations	42
		Example Problems	428
Index			44

Chapter 1

Example Problems Involving Phase Relations for Soils

1.0 GENERAL COMMENTS

Soils are prime examples of complex engineering materials, whereas in elementary physics, solid, liquid, and gaseous states are distinguished. Soils are not simple bodies that can be placed in one of these three groups. Soils are generally composed of *solid*, *liquid*, and *gas*, with the solid part being a porous medium made up of numerous particles. Soils are thus *particulate* materials.

The behavior of soils is largely determined by the *relative* amounts of the aforementioned constituents. To quantify these relative amounts requires knowledge of the "mass—volume" or "weight—volume" relations. These relations quantify a soil's *aggregate properties*.

1.1 GENERAL DEFINITIONS

The volume of the various constituents of a soil is quantified by following quantities:

 $V = \text{total volume of a soil. In some books } V_t \text{ denotes the total volume.}$

 V_{ν} = volume of the voids (pores).

 V_s = volume of the solid phase.

 V_a = volume of the gas in the voids.

 V_w = volume of the liquid in the voids.

Thus, for all soils

$$V = V_v + V_s = (V_a + V_w) + V_s \tag{1.1}$$

The mass of the various constituents of a soil is quantified by following quantities:

M = total mass of a soil. In some books M_t denotes the total mass.

 $M_a = \text{mass of the gas in the voids (pores)} = 0.$

 $M_w =$ mass of the liquid in the voids.

 M_s = mass of the solid phase.

The weight of the various constituents of a soil is quantified by following quantities:

 $W = \text{total weight of a soil. In some books } W_t \text{ denotes the total weight.}$

 W_a = weight of the gas in the voids (pores) = 0.

 W_w = weight of the liquid in the voids.

 W_s = weight of the solid phase.

Thus, for all soils

$$W = W_w + W_s \tag{1.2}$$

Remark: If $V_v = V_w (\Rightarrow V_a = 0)$ and $W_w \neq 0$, the soil is said to be *saturated*; otherwise it is *unsaturated*.

A very convenient, although somewhat idealized, way in which to visualize the mass—volume and weight—volume relations is through the use of phase diagrams. A phase diagram depicts the three phases of a soil as being segregated. For example, Figure 1.1 shows a phase diagram that relates the volume and mass of the three phases.

Figure 1.2 shows a similar phase diagram that relates the volume and weight of the three phases.

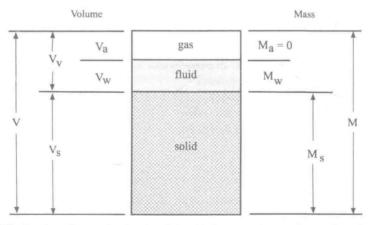


FIGURE 1.1 Phase diagram showing the relationship between volume and mass of gas, fluid, and solid phases in a soil.

此为试读,需要完整PDF请访问: www.ertongbook.com

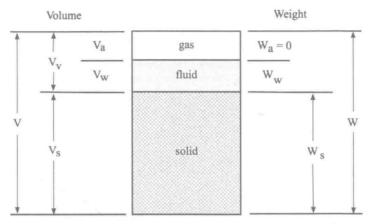


FIGURE 1.2 Phase diagram showing the relationship between volume and weight of gas, fluid, and solid phases in a soil.

1.2 MASS DENSITIES

The following mass densities are used to quantify the relative amounts of a soil's constituents:

Soil (moist) mass density:

$$\rho = \frac{M}{V} \tag{1.3}$$

Solid mass density:

$$\rho_s = \frac{M_s}{V_s} \tag{1.4}$$

Dry mass density:

$$\rho_d = \frac{M_s}{V} \tag{1.5}$$

Mass density of water:

$$\rho_w = \frac{M_w}{V_w} \tag{1.6}$$

At 4° C, $\rho_w = \rho_0 = 1000 \text{ kg/m}^3 = 1 \text{ g/cm}^3 = 1 \text{ Mg/m}^3 = 1.941 \text{ slug/ft}^3$. For ordinary engineering applications at other temperatures, $\rho_w \cong \rho_0$.

1.3 UNIT WEIGHTS

The following unit weights are used to quantify the relative amounts of a soil's constituents:

• Soil (moist) unit weight:

$$\gamma = \frac{W}{V} = \frac{Mg}{V} = \rho g \tag{1.7}$$

Solid unit weight:

$$\gamma_s = \frac{W_s}{V_s} = \frac{M_s g}{V_s} = \rho_s g \tag{1.8}$$

• Dry unit weight:

$$\gamma_d = \frac{W_s}{V} = \frac{M_s g}{V} = \rho_d g \tag{1.9}$$

Unit weight of water:

$$\gamma_w = \frac{W_w}{V_w} \tag{1.10}$$

At 4°C, $\gamma_w = \gamma_0 = 9810 \text{ N/m}^3 = 9.81 \text{ kN/m}^3 = 62.4 \text{ lb/ft}^3$. For ordinary engineering applications at other temperatures, $\gamma_w \approx \gamma_0$. In the equations, $g = 9.81 \text{ m/s}^2 = 32.2 \text{ ft/s}^2$ is the gravitational acceleration.

1.4 DEFINITION OF FUNDAMENTAL QUANTITIES

The specific gravity of solids is defined as follows:

$$G_s = \frac{\gamma_s}{\gamma_0} \approx \frac{\gamma_s}{\gamma_w} = \frac{W_s}{V_s \gamma_w} \tag{1.11}$$

Remark: G_s normalizes the solid unit weight of a material.

The volume of voids is defined by two quantities, namely the porosity n, and the void ratio e, where,

$$n = \left(\frac{V_{\nu}}{V}\right) * 100\% \tag{1.12}$$

and

$$e = \frac{V_{\nu}}{V_c} \tag{1.13}$$

The relative weight and volume of the pore fluid is quantified by the moisture content (w) and the degree of saturation (S), where,

$$w = \left(\frac{W_w}{W_s}\right) * 100\% \tag{1.14}$$

and

$$S = \left(\frac{V_w}{V_v}\right) * 100\% \tag{1.15}$$

For a saturated soil, $V_w = V_v$ and S = 100%.

1.5 RELATIONS DERIVED FROM FUNDAMENTAL **QUANTITIES**

The basic quantities G_s , n, e, w, and S can be suitably combined to form relations that are particularly useful for particular types of problems. These relations do not, however, constitute any new definitions of quantities used to describe the phase relations for soils. Some specific examples of such relations are given in the following section.

1.5.1 Case 1.1: Relation Between Void Ratio and Porosity

Rewriting the void ratio definition in terms of the volume of voids (V_v) and then dividing through by the total volume (V) gives the following relation:

$$e = \frac{V_{\nu}}{V_{s}} = \frac{V_{\nu}}{V - V_{\nu}} = \frac{V_{\nu}/V}{1 - (V_{\nu}/V)} = \frac{n}{1 - n}$$
(1.16)

where n is understood to be a decimal number.

1.5.2 Case 1.2: Relation Between Porosity and Void Ratio

Rewriting the porosity definition by expanding the total volume (V) and then dividing through by the volume of solids (V_s) gives the following relation:

$$n = \left(\frac{V_{\nu}}{V}\right) * 100\% = \left(\frac{V_{\nu}}{V_{s} + V_{\nu}}\right) * 100\% = \left(\frac{V_{\nu}/V_{s}}{1 + V_{\nu}/V_{s}}\right) * 100\%$$

$$= \left(\frac{e}{1 + e}\right) * 100\%$$
(1.17)

This result could likewise have been obtained by solving the equation derived in Case 1.1 for porosity in terms of void ratio.

1.5.3 Case 1.3: Relation Between Moisture Content, Specific Gravity of Solids, Void Ratio, and Degree of Saturation

The weight of the solid phase is written in terms of G_s as follows:

$$G_s = \frac{\gamma_s}{\gamma_w} = \frac{W_s}{V_s \gamma_w} \Rightarrow W_s = G_s V_s \gamma_w$$
 (1.18)

Next, the weight of the pore fluid is written in terms of γ_w .

$$\gamma_w = \frac{W_w}{V_w} \Rightarrow W_w = V_w \gamma_w \tag{1.19}$$

Substituting Eqs. (1.18) and (1.19) into the definition of the moisture content (Eq. 1.14) gives,

$$w = \left(\frac{W_w}{W_s}\right) * 100\% = \left(\frac{V_w \gamma_w}{G_s V_s \gamma_w}\right) * 100\% = \left(\frac{V_w}{G_s V_s}\right) * 100\%$$
 (1.20)

The volume of pore fluid is next written in terms of the degree of saturation; i.e.,

$$S = \left(\frac{V_w}{V_v}\right) * 100\% \Rightarrow V_w = \left(\frac{S}{100\%}\right)V_v = \left(\frac{S}{100\%}\right)eV_s \tag{1.21}$$

where the definition of the void ratio has been used. Substituting Eq. (1.21) into Eq. (1.20) gives the desired relation; i.e.,

$$w = \frac{Se}{G_s} \quad \text{or} \quad Se = G_s w \tag{1.22}$$

where w and S are understood to be decimal numbers.

The aforementioned expression shows that the moisture content (w) is thus a function of three quantities, namely e, S, and G_s . The upper bound on w corresponds to the case of full saturation (i.e., S = 100%), when $w \equiv w_{sat} = e/G_s$. The lower bound on w is zero, which corresponds to a completely dry soil for which S = 0%.

1.5.4 Case 1.4: Relation Between Dry Unit Weight, Specific Gravity of Solids, and Void Ratio

Beginning with the definition of the dry unit weight given by Eq. (1.9), substituting for W_s in terms of G_s gives,

$$\gamma_d = \frac{W_s}{V} = \frac{G_s V_s \gamma_w}{V_s + V_v} \tag{1.23}$$

Dividing through the resulting expression by V_s gives the desired relation; i.e.,

$$\gamma_d = \frac{G_s \gamma_w}{1 + e} \tag{1.24}$$

1.5.5 Case 1.5: Relation Between Moist Unit Weight, Specific Gravity of Solids, Moisture Content, and Void Ratio

Beginning with the definition of the moist unit weight given by Eq. (1.7), and representing the weight of the pore fluid in terms of the w and W_s gives

$$\gamma = \frac{W}{V} = \frac{W_s + W_w}{V_s + V_v} = \frac{W_s(1+w)}{V_s + V_v}$$
(1.25)

where w is understood to be a decimal number. Substituting for W_s in terms of G_s (i.e., $W_s = G_s V_s \gamma_w$) and diving through the resulting expression by V_s gives the desired relation; i.e.,

$$\gamma = \frac{G_s \gamma_w (1+w)}{1+e} \tag{1.26}$$

1.5.6 Case 1.6: Relation Between Moist Unit Weight, Dry Unit Weight, and Moisture Content

In light of Eq. (1.24), the relation for γ derived in of Case 1.5 becomes

$$\gamma = \gamma_d (1+w)$$
 or $\gamma_d = \frac{\gamma}{(1+w)}$ (1.27)

where w is understood to be a decimal number.

1.5.7 Case 1.7: Relation Between Moist Unit Weight, Specific Gravity of Solids, Degree of Saturation, and Void Ratio

Replacing the moisture content in Eq. (1.26) with the relation derived in Case 1.3 (i.e., $w = Se/G_s$) gives

$$\gamma = \frac{\gamma_w(G_s + Se)}{1 + e} \tag{1.28}$$

1.5.8 Case 1.8: Unit Weight of Submerged Soil and Its Relation to Moist Unit Weight

Consider a saturated soil that is submerged in water. According to Archimedes' principle, the buoyancy force acting on a body is equal to the weight of the fluid displaced by the body.

Since the soil is saturated, S = 100% and $V_w = V_v$. The buoyant unit weight is thus

$$\gamma_b = \frac{(W_s - V_s \gamma_w) + (W_w - V_v \gamma_w)}{V_s + V_v}$$
 (1.29)

Writing W_s in terms of G_s and W_w in terms γ_w gives

$$\gamma_b = \frac{(G_s V_s \gamma_w - V_s \gamma_w) + (V_v \gamma_w - V_v \gamma_w)}{V_s + V_v} = \frac{\gamma_w V_s (G_s - 1)}{V_s + V_v}$$
(1.30)

Dividing through the equation by V_s gives the final expression for the buoyant unit weight; i.e.,

$$\gamma_b = \frac{\gamma_w(G_s - 1)}{1 + e} \tag{1.31}$$

For a saturated soil the expression for moist unit weight given by Eq. (1.28) reduces to

$$\gamma = \gamma_{sat} = \frac{\gamma_w(G_s + e)}{1 + e} \tag{1.32}$$

Manipulating this expression gives the relationship between the saturated and buoyant unit weights; i.e.,

$$\gamma_{sat} = \frac{\gamma_w(G_s + e)}{1 + e} = \frac{\gamma_w(G_s - 1)}{1 + e} + \frac{\gamma_w(1 + e)}{1 + e} = \gamma_b + \gamma_w$$
 (1.33)

or

$$\gamma_b = \gamma_{sat} - \gamma_w \tag{1.34}$$

EXAMPLE PROBLEM 1.1

General Remarks

Knowing the definitions of the basic quantities e, n, w, S, and G_s , it is relatively straightforward to derive more specific relations than those presented in Cases 1.1 - 1.8.

Problem Statement

Derive an expression for void ratio (e) in terms of the total weight (W), total volume (V), the unit weight of water (γ_w) , the degree of saturation (S), and the specific gravity of solids (G_s) .

Solution

Recall the relation for moist unit weight derived in Case 1.7 (Eq. 1.28); i.e.,

$$\gamma = \frac{W}{V} = \frac{\gamma_w(G_s + Se)}{1 + e} \tag{1.1.1}$$

Solving for the void ratio leads to the following results:

此为ett,
$$(V\gamma_w)$$
 $(G_s \pm Se)$ $\Rightarrow e(1-\frac{V}{W}\gamma_w S) = \frac{V}{W}\gamma_w G_s - 1$ $(1.1.2)$ com