Structural Analysis

A UNIFIED CLASSICAL AND MATRIX APPROACH

A. GHALI

A. M. NEVILLE

SEC IDITION

Structural Analysis

A UNIFIED CLASSICAL AND MATRIX APPROACH

A. GHALI

Professor of Civil Engineering University of Calgary

A. M. NEVILLE

Principal and Vice-Chancellor, University of Dundee

Chapters 20 and 24 written in collaboration with the authors by Y. K. Cheung
Professor of Civil Engineering,
University of Hong Kong

SECOND EDITION

LONDON NEW YORK
CHAPMAN AND HALL

First published 1972
by Intext Educational Publishers
Second edition 1978
published by
Chapman and Hall Ltd
11 New Fetter Lane, London EC4P 4EE
Published in the USA by
Chapman and Hall
733 Third Avenue, New York NY 10017
Reprinted 1979, 1983

© 1978 A. Ghali and A. M. Neville

Printed in Great Britain by Richard Glay (The Chaucer Press) Ltd Bungay, Suffolk

ISBN 0 412 14990 7

This paperback edition is sold subject to the condition that it shall not, by way of trade or otherwise, be lent, re-sold, hired out, or otherwise circulated without the publisher's prior consent in any form of binding or cover other than that in which it is published and without a similar condition including this condition being imposed on the subsequent purchaser.

All rights reserved. No part of this book may be reprinted, or reproduced or utilized in any form or by any electronic, mechanical or other means, now known or hereafter invented, including photocopying and recording, or in any information storage or retrieval system, without permission in writing from the publisher

Preface to the Second Edition

Since this book is concerned with analysis, rather than design, the main text does not involve any specific system of units. The same applies to most of the problems set for the reader to solve. However, there are a number of problems where it was thought advantageous to use actual dimensions of members and to specify the magnitude of forces. These problems are set both in the so-called British units (common in North America) and in the SI units (now becoming accepted all over the world). The former version appears at the end of the various chapters and the latter, bearing the same number, is given in a separate section at the end of the book. It is up to the reader to choose which set of problems he wants to solve—and of course the underlying theory is the same. Naturally, answers to all the problems are listed.

We would like to take this opportunity to thank those readers, especially students at the University of Calgary, who assisted in correcting errors in the answers to problems in the first edition. While to expect that no errors are left would be too optimistic, we hope that their number is tending to zero.

A. Ghali A M. Neville

Calgary, Alberta, Canada Leods, England July 1977

Preface to the First Edition

This book has been written so that it can be followed by anyone familiar with the application of basic mechanics to statically determinate structures—material usually covered in early courses in engineering. The material presented is both elementary and advanced, the emphasis being on a unified presentation of the whole field of structural analysis. Indeed, the book combines the classical and modern methods of analysis of statically indeterminate structures.

The text has been developed in teaching, over a number of years, undergraduate and graduate courses at the University of Calgary, Canada. The first fourteen chapters contain basic material which should be covered in the first course or courses; from the remainder of the book, a suitable choice, depending on requirements, can be made to form a more advanced course. This arrangement was chosen in order to make the book suitable not only for the student but also for the practicing engineer who wants to advance his knowledge by independent study and to obtain guidance on the most convenient methods of analysis of a variety of types of structures.

The methods of analysis introduced in the various chapters are illustrated by fully solved examples in text. At the end of each chapter carefully selected, instructive problems are set, with answers at the end of the book. Data frequently used are presented in appendices, one of which offers a full introduction to matrix algebra for the benefit of those not already familiar with it.

Matrix algebra is particularly useful in structural analysis because it makes possible a formulation of the solution as a series of matrix operations suitable for a digital computer. But even more important is that, by using matrices, structures of all types can be analyzed through a general approach, and because of the organizational properties of matrices, the use of matrix algebra is advantageous even in hand computation. Matrix formulation makes it possible to represent equations in a compact form: this not only saves space but also helps the reader to concentrate on the overall operations without being distracted by algebraic or arithmetical details. For all these reasons the matrix notation is extensively used in the book.

In the first four chapters we introduce two distinct general approaches of analysis: the force method and the displacement method. In the former, the structure is made statically determinate by the removal of restraining forces; a solution satisfying statics is then obtained, and the corresponding inconsistency in geometry is corrected by additional forces. In the displacement

method, artificial restraints are added and the corresponding restraining forces are calculated: the equilibrium conditions are then satisfied by the removal of the restraining forces, thus allowing the displacements to occur and returning the structure to its actual condition. Throughout the book emphasis is placed on an overall unifying view, with these two distinct approaches in mind.

Both the force and displacement methods involve the solution of linear simultaneous equations relating forces to displacements. The emphasis in the first chapters is on the basic ideas in the two methods without obscuring the procedure by the details of derivation of the coefficients needed to form the equations. Instead, use is made of Appendices B, C, and D which give displacements due to applied unit forces, forces corresponding to unit displacements, and fixed-end forces due to various loadings. The consideration of the detailed methods of displacement calculation is thus delayed to Chapters 5 to 9, by which time the need for this material in the analysis of statically indeterminate structures is clear. This sequence of presentation of material is particularly suitable when the reader is already acquainted with some of the methods of calculating the deflection in beans. If, however, it is thought preferable first to deal with methods of calculation of displacement, Chapters 5 to 9 should be read before Chapters 2 to 4; this will not disturb the continuity.

The analysis of continuous beams, frames, and trusses is treated in Chapters 10 to 12. These include the methods of column analogy, slopedeflection and moment distribution, and are presented as applications of the force or displacement methods. The numerous solved examples included vary from simple problems to the more complicated types of structures of practical occurrence. Some techniques to speed up the analysis of building frames of several storeys or several bays are discussed in Chapter 12. In essence, these are the classical methods suitable for hand computation but their value to the structural designer is undiminished for preliminary calculations, for checking computer results, and of course when no computer is available.

Influence lines and influence coefficients are a useful means of analysis of structures subjected to moving loads or to loadings which can have varying positions. Chapters 13 and 14 deal with the methods of obtaining influence lines; Müller-Breslau's principle is explained and applied to different types of structures, including gridworks and interconnected bridge systems. Chapter 13 and the major part of Chapter 14 are concerned with influence lines which represent the effect of a unit gravity load. In the remainder of Chapter 14, influence lines for the effect of two other types of loading are discussed; one of these influence lines is applied in the analysis of statically indeterminate prestressed structures.

The effect of axial forces in framed structures is considered in Chapter 15.

Both the effect of the change in length and of the change in stiffness characteristics are discussed and applied to the problem of secondary stresses in trusses and in the determination of the critical buckling loads of continuous frames.

Chapter 16 deals with the analysis of shear walls, commonly used in modern buildings. The chapter summarizes the present knowledge, states the simplifying assumptions usually involved and presents a method of analysis that can be applied in most practical cases.

With the present availability of computers, numerical methods are increasingly used in the practical analysis of structures. The finite-difference method and, to a larger extent, the finite-element method are powerful tools which involve a large amount of computation. Chapter 17 deals with the use of finite differences in the analysis of structures composed of beam elements and extends the procedure to axi-symmetrical shells of revolution. In Chapter 18 the finite-difference method is used in the analysis of plates and in Chapter 19 it is applied to obtain influence, flexibility, and stiffness coefficients useful in a variety of cases.

The finite-element method for the analysis of continuum problems is based on the energy theorems and on the displacement method of analysis, applied to framed structures in earlier chapters. For this reason, it is possible to represent in Chapter 20 the basic general equations with a wide range of applications. Sufficient details of the finite-element method are included for the solution of problems in plane elasticity and in bending of plates. The finite-strip method discussed in the latter part of Chapter 20 is an extension of the finite-element approach which offers some saving in the amount of computation.

Modern design of structures is based on both the elastic and plastic analyses. The plastic analysis cannot replace the elastic analysis but supplements it by giving useful information about the collapse load and the mode of collapse. Chapters 21 and 22 deal with the plastic analysis of framed structures and slabs respectively.

An introduction to structural dynamics is presented in Chapter 23. This is a study of the response of structures to dynamic loading produced by machinery, gusts of wind, blast, or earthquakes. First, free and forced vibrations of a system with one degree of freedom are discussed. This is then extended to multidegree-of-freedom systems, taking advantage of the matrix approach.

In Chapter 24 some computational techniques in the analysis of large structures are discussed. The basic ideas are explained without details of computer programming, for which reference should be made to specialized manuals or texts. But the present book, coupled with practice in computer programming, gives sufficient knowledge to solve a wide variety of problems in structural analysis.

x Preface to the First Edition

For the convenience of the American and Canadian readers, and of many British, British units are used in numerical examples. However, this should not cause difficulty for other readers as the book is concerned with methods of analysis rather than with design, and all the derivations and expressions are written in symbols.

The authors wish to thank Mr. G. S. Tadros and Mr. B. Langan of the University of Calgary for providing answers to most of the problems; numerous past students of the same university for solution of many problems; Dr. J. Harrop of the University of Leeds for his helpful collaboration in writing Chapter 22; and Dr. Y. K. Cheung of the University of Calgary for criticism and suggestions on some parts of the text, as well as for his work on Chapters 20 and 24, acknowledged on the title page. The typing of the manuscript was carefully done by Miss K. M. F. Dallas, Miss S. M. Fawcett, Mrs. C. I. Hills, Mrs. J. M. Misaelides, and Mrs. G. Donaldson and the manuscript was painstakingly proofread by Mr. P. K. Das of the University of Leeds.

A. Ghali
A. M. Neville

Calgary, Canada Leeds, England September, 1971

Notation

The following is a list of symbols which are common in the various chapters of the text; other symbols are used in individual chapters. All symbols are defined in the text when they first appear.

A Any action, which may be a reaction or a stress resultant. A stress

resultant at a section of a framed structure is an internal force:

bending moment, shearing force or axial force.

a Cross-sectional area.

 D_i or D_{ij} Displacement (rotational or translational) at coordinate i. When a

second subscript j is provided it indicates the coordinate at which

the force causing the displacement acts.

E Modulus of elasticity.

EI Flexural rigidity.

F A generalized force: a couple or a concentrated load.

FEM Fixed-end moment.

f_{ij} Element of flexibility matrix.G Modulus of elasticity in shear.

I Moment of inertia.

i, j, k, m, n, p, r Integers.

J Torsion constant (length⁴), equal to the polar moment of inertia for

a circular cross section

Length.

M Bending moment at a section. In beams and grids, a bending moment

is positive when it causes tension in bottom fibers.

 M_{AB} Moment at end A of member AB. In plane structures, an end-moment

is positive when clockwise. In general, an end-moment is positive when it can be represented by a vector in the positive direction of the

axes x, r, or z.

N Axial force at a section or in a member of a truss.

P, Q Concentrated loads.
q Load intensity.
R Reaction.

 S_{ij} Element of stiffness matrix.

Used as a subscript indicates a statically determinate action.

T Twisting moment at a section.

Used as a subscript, indicates the effect of unit forces or unit displace-

ments

1 Shearing force at a section.

xxii Notation

W	Work of the external applied forces.
3	Strain.
η	Influence ordinate.
v	Poisson's ratio.
σ	Stress.
τ	Shearing stress.
()	Braces indicate a vector, i.e., a matrix of one column. To save space, the elements of a vector are sometimes listed in a row between two braces.
[]	Brackets indicate a rectangular or square matrix.
$\begin{bmatrix} & \end{bmatrix}_{n \times m}^{T}$	Superscript T indicates matrix transpose. $n \times m$ indicates the order of the matrix which is to be transposed resulting in an $m \times n$ matrix.
	Double-headed arrow indicates a couple or a rotation; its direction is that of the rotation of a right-hand screw progressing in the direction of the arrow.
	Single-headed arrow indicates a load or a translational displacement.
$Z_{\uparrow} \longrightarrow X$	Axes: the positive direction of the z axis points away from the reader.

The SI System of Units of Measuremen

Length	metre				m
	millimetre	$= 10^{-3}$ n	n		mm
Area	square me	tre			m^2
	square mi	llimetre =	$= 10^{-6} \text{m}^2$		mm ²
Volume	cubic met	re			m^3
Frequency	hertz = 1	cycle per	second		Hz
Mass	kilogram	,			kg
Density	kilogram j	per cubic	metre		kg/m³
Force	newton				N .
 a force which applied to a mass of one kilogram gives it an acceleration of one metre per second per second, i.e. 1N = 1kg m/s² 					
Stress	newton per square metre			N/m^2	
newton per square millimetre				N/mm^2	
Temperature interval	degree Cel	sius			deg C; °C
Nomenclature	Nomenclature for multiplication factors				
		10 ⁹ 10 ⁶ 10 ³ 10 ⁻³ 10 ⁻⁶ 10 ⁻⁹	giga mega kilo milli micro nano	G M k m μ n	,

XXIII

Contents

Notat	ion		XX1
	CHAPTER 1		
Introd	luction to the Analysis of Statically Indeterminate Structures		1
1-1	INTRODUCTION		1
1-2	STATICAL INDETERMINACY		
1-3	EXPRESSIONS FOR DEGREE OF INDETERMINACY		
1-4	GENERAL METHODS OF ANALYSIS OF STATICALLY IN-		
	DETERMINATE STRUCTURES	4 2 2 XXXXXX	12
1-5	KINEMATIC INDETERMINACY		13
1-6	PRINCIPLE OF SUPERPOSITION		
1-7	GENERAL		20
	PROBLEMS		21
	CHAPTER 2		
Force	Method of Analysis	180	24
2-1	INTRODUCTION		24
2-2	DESCRIPTION OF METHOD		24
2-3	FLEXIBILITY MATRIX		26
2-4	ANALYSIS FOR DIFFERENT LOADINGS		29
2-41	Effect of Displacement at Joints: Environmental Effects		
2-42	Effect of Displacement at Coordinates		31
2-5	EQUATION OF THREE MOMENTS		37
2-6	GENERAL		41
	PROBLEMS		42
	CHAPTER 3		
Disple	acement Method of Analysis		45
3-1	INTRODUCTION		
3-2	DESCRIPTION OF METHOD		
3-3	STIFFNESS MATRIX		
3-4	ANALYSIS FOR DIFFERENT LOADINGS		
3-5	ANALYSIS FOR ENVIRONMENTAL EFFECTS		
3-6	EFFECT OF DISPLACEMENTS AT COORDINATES		64
3-7	GENERAL		65
	PROBLEMS		
	CHAPTER 4		
Flexi	bility and Stiffness Matrices		71
4-1	bility and Stiffness Matrices INTRODUCTION	1	71
4-2	RELATION BETWEEN FLEXIBILITY AND STIFFNESS		₹ 5
	MATRICES		71

"2.P" W 45	Contents
XII	

4-3 4-4 4-5 4-6 4-7	CHOICE OF FORCE OR DISPLACEMENT METHOD	
	CHAPTER 5	
Strain	Energy and Virtual Work	90
5-1	INTRODUCTION	91
5-2	GEOMETRY OF DISPLACEMENTS	
5-3	STRAIN ENERGY	
5-31	Strain Energy Due to Axial Force	98
5-32	Strain Energy Due to Bending Moment	98
5-33	Strain Energy Due to Shear	
5-34	Strain Energy Due to Torsion	101
5-35	Total Strain Energy	102
5-4	COMPLEMENTARY ENERGY AND COMPLEMENTARY .	
	WORK	102
5-5	PRINCIPLE OF VIRTUAL WORK	104
5-6	UNIT LOAD AND UNIT DISPLACEMENT THEOREMS	
5-7	VIRTUAL WORK TRANSFORMATIONS	
5-8	GENERAL	110
Sev sup	CHAPTER 6	
Methoa	of Virtual Work and its Application to Trusses INTRODUCTION	112
6-1	INTRODUCTION	112
6-2	CALCULATION OF DISPLACEMENT BY VIRTUAL WORK	
6-3	DISPLACEMENT OF STATICALLY INDETERMINATE	
	STRUCTURES	114
6-4	EVALUATION OF INTEGRALS FOR CALCULATION OF	
	DISPLACEMENT BY METHOD OF VIRTUAL WORK	116
6-5	TRUSS DEFLECTION	
6-6	TRUSS DEFLECTION USING MATRIX ALGEBRA	122
6-7	GENERAL	124
	PROBLEMS	124
*	· ·	
	CHAPTER 7	9 0100
Further	Applications of Method of Virtual Work	127
7-1	INTRODUCTION	
7-2	EQUIVALENT JOINT LOADING	
7-3	DEFLECTION OF BEAMS AND FRAMES	129
7-4	DEFLECTIONS OF BEAMS AND FRAMES USING MATRIX	
• •	ALGEBRA	
7-5	FLEXIBILITY MATRIX OF THE ASSEMBLED STRUCTURE	
7-6	GENERAL	
	PROBLEMS	147
	CHAPTER 8	

8-1	INTRODUCTION	151
8-2	BETTI'S AND MAXWELL'S THEOREMS	121
8-3	APPLICATION OF BETTI'S THEOREM TO TRANSFORMA-	153
	TION OF FORCES AND DISPLACEMENTS	
8-4	TRANSFORMATION OF STIFFNESS AND FLEXIBILITY	158
	MATRICES	160
8-5	STIFFNESS MATRIX OF ASSEMBLED STRUCTURE	163
8-6	ENGESSER'S THEOREM OF COMPATABILITY	166
8-7	CASTIGLIANO'S THEOREM OF COMPATABLETT	
8-8	CALCULATION OF DISPLACEMENT BY COMPLEMENTARY ENERGY	166
W IN	CLASTICI LA NOS THEOREMS	107
8-9	POTENTIAL ENERGY	170
8-10	CINICDAT	1/2
8-11	PROBLEMS	172
	CHAPTER 9	
Displac	ement of Elastic Structures by Special Methods	177
9-1	INTRODUCTION	177
0. 2	CRADUICAL DETERMINATION OF DEFLECTION OF A	
	PLANE TRUSS	177
9.7	DIFFERENTIAL FOLIATION FOR DEFLECTION OF A BEAM	
3,-8.	IN RENDING	180
0.4	MOMENT-AREA THEOREMS	182
0.5	METHOD OF FLASTIC WEIGHTS	186
9-31	Fanivalent Concentrated Loading	190
9-6	METHOD OF FINITE DIFFERENCES	194
9-7	REPRESENTATION OF DEFLECTIONS BY FOURIER SERIES.	196
9-8	DEPRESENTATION OF DEFLECTIONS BY SERIES WITH	
	INDETERMINATE PARAMETERS	198
9-9	GENERAL	204
	PROBLEMS	20:
	CHAPTER 10	
		210
Applic	ation of the Force Method: Column Analogy	277,1000
10-1	INTRODUCTION.	717
10-2	ELASTIC CENTER AND ANALOGOUS COLUMN	21
10-23	Unsymmetrical Bents CHOICE OF THE RELEASED STRUCTURE	21
10-3	CHOICE OF THE RELEASED STRUCTURE	22
1()-1	END-STIFFNESS OF PLANE FRAMES End-Rotation	22
10-41	End-Translation	22
10-4	End-Translation End-Forces	22
10-42	Ind-Forces	22
10-44	Lorced Displacement	
10-5	CONDITIONS	22
(E)00 E	ENDISTIFFNESS OF STRAIGHT MEMBERS	22
1()-6	CORRECTION FOR THE EFFECT OF AXIAL FORCES IN	
10-7	ARCHES	23
10 =	GENERAL.	21
10-8	PROBLEMS	23

CHAPTER 11

	ation of the Displacement Method: Stope-Deflection and	
Mome	ent Distribution	241
11-1	INTRODUCTION	241
11-2	NOTATION AND SIGN CONVENTION	242
11-3	SLOPE-DEFLECTION EQUATION FOR A STRAIGHT	8
	MEMBER	243
11-4	EQUATIONS OF EQUILIBRIUM	
11-5	PROCESS OF MOMENT DISTRIBUTION	
11-6	MOMENT-DISTRIBUTION PROCEDURE FOR PLANE	
	FRAMES WITHOUT JOINT TRANSLATION	254
11-7	ADJUSTED END-ROTATIONAL STIFFNESSES	
11-8	ADJUSTED FIXED-END MOMENTS	
11-9	GENERAL	
		207
	CHAPTER 12	
Mome	nt Distribution with Sway: Multistorey and Multibay Frames	268
12-1	INTRODUCTION	268
12-2	GENERAL PROCEDURE FOR PLANE FRAMES WITH	2/0
12.2	JOINT TRANSLATION	
12-3		
12-4	METHOD OF SUCCESSIVE SWAY CORRECTIONS	
12-5 12-6	THE SUBSTITUTE-FRAME METHOD	
12-6	THE PRINCIPLE OF MULTIPLE	290
12-7		202
12.0	PATTERN	
12-8	GENERAL	
	PROBLEMS	295
	CHAPTER 13	
Influer	nce Lines for Beams, Frames, and Grids	303
13-1	INTRODUCTION	202
13-2	CONCEPT AND APPLICATION OF INFLUENCE LINES	303
13-3	MÜLLER-BRESLAU'S PRINCIPLE	303
13-31	Procedure for Obtaining Influence Lines	305
13-4	CORRECTION FOR INDIRECT LOADING	309
13-5	INFLUENCE LINES FOR A BEAM WITH FIXED ENDS	212
13-6	INFLUENCE LINES FOR PLANE FRAMES	312
13-7	INFLUENCE LINES FOR GRIDS	220
13-8	GENERAL	320
15 0	GENERAL	
2.5	CHAPTER 14	5
	ce Lines for Arches, Trusses, and Prestressed Concrete	
Membe	Prs	332
14-1	INTRODUCTION	332
14-2	GENERAL SUPERPOSITION EQUATION	
14-3	INFLUENCE LINES FOR ARCHES	
14-4	INFLUENCE LINES FOR TRUSSES	
14-5	PRESTRESSING MOMENT INFLUENCE COEFFICIENTS	
14-6	RELATION BETWEEN INFLUENCE LINES	
	· ·	

14-7	PRESTRESSING MOMENT INFLUENCE COEFFICIENTS		
	FOR FRAMES AND BEAMS		
14-8	PRESTRESSING MOMENT INFLUENCE COEFFICIENTS		
	FOR GRIDS		
14-9	GENERAL351		
	PROBLEMS		
	CHAPTER 15		
Effects	of Axial Forces 356		
15-1	INTRODUCTION356		
15-2	FFFCT OF CHANGE IN LENGTH		
15-3	SECONDARY MOMENTS IN TRUSSES		
15-31	Steps in Calculation by Moment Distribution		
15-32	Relative Chord Rotation		
15-4	STIFFNESS OF PRISMATIC MEMBER SUBJECTED TO AN		
	AXIAL FORCE		
15-41	Effect of Axial Compression		
15-42	Effect of Axial Tension		
15-43	General Treatment		
15-5	ADJUSTED END-ROTATIONAL STIFFNESS FOR A PRISMATIC		
	MEMBER SUBJECTED TO AN AXIAL FORCE		
15-6	FIXED-END MOMENTS FOR A PRISMATIC MEMBER		
15.0	SUBJECTED TO AN AXIAL FORCE		
15-61	Uniform Load		
15-62	Concentrated Load		
15-7	ADJUSTED FIXED-END MOMENTS FOR A PRISMATIC		
	MEMBER SUBJECTED TO AN AXIAL FORCE		
15-8	ELASTIC STABILITY OF FRAMES		
15-9	CALCULATION OF BUCKLING LOAD FOR FRAMES BY		
75 CEN 100	MOMENT DISTRIBUTION,		
15-10	GENERAL		
	PROBLEMS		
	CHAPTER 16 398		
Analys	is of Shear wans		
16-1	INTRODUCTION398		
16-2	STIFFNESS OF A SHEAR-WALL ELEMENT		
16-3	STIFFNESS MATRIX OF A BEAM WITH RIGID END PARTS402		
16-4	ANALYSIS OF A PLANE FRAME WITH SHEAR WALLS404		
16-5	SIMPLIFIED APPROXIMATE ANALYSIS OF A BUILDING		
	AS A PLANE STRUCTURE408		
16-51	Special Case of Similar Columns and Beams		
16-6	SHEAR WALLS WITH OPENINGS		
16-7	THREE-DIMENSIONAL ANALYSIS		
16-71	One-Storey Structure		
16-72	Multistorey Structure		
16-8	GENERAL431		
	PROBLEMS		
CHAPTER 17			
Metho	Method of Finite Differences 435		
112 6 6160	······································		

xvi Contents

17-1	INTRODUCTION	43.
17-2	REPRESENTATION OF DERIVATIVES BY FINITE DIFFER-	
	ENCES.	430
17-21	Errors in Finite-Difference Equations	430
17-3	BENDING MOMENTS AND DEFLECTIONS IN A STATICALLY	
	DETERMINATE BEAM	
17-4	FINITE-DIFFERENCE RELATION BETWEEN BEAM	
f. serve	DEFLECTION AND APPLIED LOADING.	4.83
17-41	Beams with a Sudden Change in Section	
17-42	Boundary Conditions	44.
17-42		44
17-3	FINITE-DIFFERENCE RELATION BETWEEN BEAM DE-	
17 (FLECTION AND STRESS RESULTANT OR REACTION	
17-6	BEAM ON AN ELASTIC FOUNDATION	450
17-7	AXISYMMETRICAL CIRCULAR CYLINDRICAL SHELL	45
17-8	CONICAL AND SPHERICAL SHELLS	
17-9	BUCKLING LOAD OF A COLUMN WITH HINGED ENDS	459
17-10	BUCKLING LOAD OF COLUMNS WITH END RESTRAINTS	465
17-11	GENERAL	467
	PROBLEMS	468
	CHAPTER 18	
4 1		
Analys	is of Plates by Finite Differences	472
18-1	INTRODUCTION	47
18-2	REPRESENTATION OF PARTIAL DERIVATIVES BY FINITE	
	DIFFERENCES	47
18-3	GOVERNING DIFFERENTIAL EQUATIONS FOR PLATES	
	SUBJECTED TO IN-PLANE FORCES	47
18-4	AIRY STRESS FUNCTION	
18-5	FINITE-DIFFERENCE EQUATIONS FOR PLATES SUB-	
	JECTED TO IN-PLANE FORCES	47"
18-51	Value of the Stress Function at Plate Boundary	401
18-6	GOVERNING DIFFERENTIAL EQUATION FOR PLATES IN	401
10.0		40.
18-7	BENDING	484
10-7		-4.07
18-8	OF A PLATE IN BENDING.	
18-9	BOUNDARY CONDITIONS OF A PLATE IN BENDING	488
	ANALYSIS OF PLATES IN BENDING.	
18-91	Stiffened Plates	496
18-10	BUCKLING OF THIN PLATES	499
18-11	GENERAL	
	PROBLEMS	503
	CHAPTER 19	
Lathern		e 21.
	ce Coefficients by Finite Differences	506
19-1	INTRODUCTION	506
19-2	COMPARISON BETWEEN EQUIVALENT STIFFNESS MATRIX	
	AND STIFFNESS MATRIX	506
19-3	INFLUENCE COEFFICIENTS OF STRESS RESULTANTS	511
19-4	TRANSFORMATION OF THE EQUIVALENT STIFFNESS	
	MATRIX	517